Traffic Impact Assessment

46-54 Court Road, Fairfield

Proposed Mixed-Use Development

24015

Prepared for

Level 33

21 February 2025



Contact Information

Genesis Traffic	Suite 3, 53 Grandview Street, Pymble
	www.genesistraffic.com.au
	ABN 34 660 055 532
Email	bernard@genesistraffic.com.au
Approved By	Bernard Lo

Document Information

Report	Traffic Impact Assessment
Proposal	Proposed Mixed-Use Development
Architect	Level 33
Project Location	46-54 Court Road, Fairfield
Council	Fairfield City Council
Job Number	24015
Date	21/02/2025

Document History

Version	Effective Date	Description of Revision	Prepared by	Reviewed by
1	20/02/2024	Draft	LN	BL
2	27/02/2024	2 nd Draft	LN	BL
3	27/02/2024	Final Draft	LN	BL
4	28/02/2024	For Issue	LN	BL
5	17/01/2025	For Issue	LN	BL
6	12/02/2025	For Issue	LN	BL
7	21/02/2025	For Issue	LN	BL



Table of Contents

1	Intr	oduction	6
	1.1	Background	6
	1.2	Scope of Works	6
	1.3	Reference Documents	7
2	Dev	relopment Scheme Context	8
	2.1	Approved Development	8
	2.2	S4.55 Modification	8
	2.3	Proposed Development	8
3	Exis	ting Conditions	10
	3.1	Site and Surrounding Context	10
	3.2	Road Network	11
	3.3	Traffic Controls	12
	3.4	Public Transport Services	13
4	Parl	king Assessment	15
	4.1	Car Parking Requirement	15
	4.2	Bicycle Parking and End of Trip Facility Requirement	16
	4.3	Loading and Servicing Requirement & Arrangement	16
5	Acc	ess and Circulation Design	18
	5.1	Access, Design Assessment and Internal Circulation	18
6	Traf	fic Assessment	19
	6.1	Existing Traffic Conditions	19
	6.2	Approved Outcome Traffic Generation & Impact	20
	6.3	Development Traffic Generation	21
	6.4	Overall Traffic Generation and Distribution	21
7	Con	clusion	27



24

Attachments

Figure 6-3

Attachment 1 Approved DA Plan

Attachment 2 Proposed Plan Attachment 3 Turning Path Assessment Attachment 4 Traffic Survey Attachment 5 SIDRA Result **Tables** Table 3-1 Surrounding Road Network 11 Table 3-2 **Surrounding Traffic Controls** 12 Table 3-3 **Bus Services Provision** 13 Table 4-1 Car Parking Rates from SEPP 2021 15 Table 4-2 Required Car Parking Spaces 15 Table 4-3 **Bicycle Parking Rates** 16 Table 4-4 Required Bicycle Storage/Parking Spaces 16 Table 6-1 Intersection Performance - Levels of Service 19 Table 6-2 **Existing Intersection Traffic Circumstance** 20 Table 6-3 Approved Traffic Generation Outcome 20 Table 6-4 Peak Hour Traffic Generation 21 Table 6-5 Net Peak Hour Traffic Generation 21 Table 6-6 Proportion of Inbound and Outbound Traffic 22 Table 6-7 Existing and Post-Development SIDRA Assessment Outcome 26 **Figures** Figure 1-1 Site 6 Figure 3-1 Site Context 10 Figure 3-2 Road Network 11 **Local Public Transport Locations** Figure 3-3 13 Figure 6-1 Approach Route Distribution 22 Figure 6-2 **Depart Route Distribution** 23

24015 | 21 February 2025 4

Inbound and Outbound Traffic during AM Peak



Figure 6-4 Inbound and Outbound Traffic during PM Peak

25





1 Introduction

1.1 Background

This report has been prepared to accompany an Amended Development Application to Fairfield City Council for a Proposed Mixed-Use Development at 46-54 Court Road, Fairfield (Figure 1-1).

Figure 1-1 Site



Source: Metromap (Modified by Genesis Traffic)

1.2 Scope of Works

The purpose of this report is to:

- describe the approved and proposed development schemes
- describe the existing site, road network serving the site and the prevailing traffic conditions
- assess the adequacy of the proposed parking provision
- assess the potential traffic implications
- assess the suitability of the proposed vehicle access, internal circulation and servicing arrangements



1.3 Reference Documents

Reference has been made to the following documents when preparing this report:

- Fairfield City Centre Development Control Plan (Fairfield City Council, 2013)
- Guide to Traffic Generating Developments, RMS, 2002
- Guide to Traffic Generating Developments, Updated Traffic Surveys, RMS, TDT 2013/14a
- State Environmental Planning Policy (Housing) 2021



2 Development Scheme Context

2.1 Approved Development

The site is previously subject to a development consent (DA 687.1/2014) that permits the following development outcome on the site:

- 290 apartments in the following composition:
 - 9 x studio apartment(s)
 - 92 x one-bedroom apartment(s)
 - 152 x two-bedroom apartment(s)
 - 37 x three-bedroom apartment(s)
- Basement carpark 434 car spaces
- Ground floor commercial/retail premises of 1,413m²

Details of the approved DAs are provided on the architectural plans in **Attachment 1**.

2.2 S4.55 Modification

It is noted that the approved scheme is the subject of a modification application (S4.55 Mod) currently being assessed by Council. Relevantly, the S4.55MOD sought consent for:

- Reduced retail from 1,413 to 1,107m² GFA
- Amended basement layout comprising 441 spaces, including 2 additional small car spaces.

For clarity, the subject amended DA will be assessed on the basis of the proposed S4.55MOD plans/layout.

2.3 Proposed Development

The proposal seeks consent for a development outcome under an Amended DA. The amended DA seeks:

- 356 apartments (under SEPP provisions) in the following composition:
 - o 57 x affordable units
 - · 30 x studio/one-bedroom apartment(s)
 - · 21 x two-bedroom apartment(s)
 - 6 x three-bedroom apartment(s)
 - 299 x non affordable units
 - 92 x studio/one-bedroom apartment(s)
 - · 170 x two-bedroom apartment(s)



- · 37 x three-bedroom or more apartment(s)
- Basement carpark 434 car spaces
- Commercial/retail premises' floor area will be reduced to 1056.69m²

The initially approved vehicle access provisions on Court Road will be maintained.

Details of proposed amended DA scheme are indicated in the architectural plans in **Attachment 2**.



3 Existing Conditions

3.1 Site and Surrounding Context

The development site (Figure 3-1) is legally known as Lot 101 in DP 717004, Lot 3 and 4 in DP 524149, located at 46-54 Court Road, Fairfield. The site occupies an area of 9,239m² and has frontage(s) to Court Road and The Horsley Drive.

Figure 3-1 Site Context



Source: Metromap (Modified by Genesis Traffic)

The existing site is vacant with vehicle access point(s) located at Court Road.

The adjoining and surrounding land uses include:

- adjoining McDonald's and residential-based mix use complexes to the north
- adjoining KFC and commercial premises to the south
- Fairfield City Central Shopping Mall opposite Court Road
- industrial premises opposite The Horsley Drive



3.2 Road Network

The existing road network serving the site area (Figure 3-2) are detailed in Table 3-1:

Figure 3-2 Road Network



Source: TfNSW (modified by Genesis Traffic)

Table 3-1 Surrounding Road Network

Road Name	Descriptions
	· State Road
	· Speed limit 60km/h
The Horsley Drive	· 2 lane(s) in each direction in general
	· Clearway restriction between 6am - 7pm Monday to Friday and 9am – 6pm Saturday, Sunday and Public Holidays along both sides of the street
	· Local Road
Nelson Street	· Speed limit 40km/h
recison server	· 2 lane(s) in each direction
	· No Parking restriction along both sides of the street
	· Local Road
Court Road	· Speed limit 40km/h
	 1 lane(s) in each direction generally except the northbound direction along the frontage of Shopping Centre that has 2 lanes



	 No Parking restriction along both sides of the street along the frontage of Shopping Centre
	· Local Road
Alan Street	· Speed limit 40km/h
	· 1 lane(s) in each direction
	· Time restricted (1P) on-street parking along both sides of the street

3.3 Traffic Controls

The traffic controls on the road system in the vicinity of the site comprise (Table 3-2):

Table 3-2 Surrounding Traffic Controls

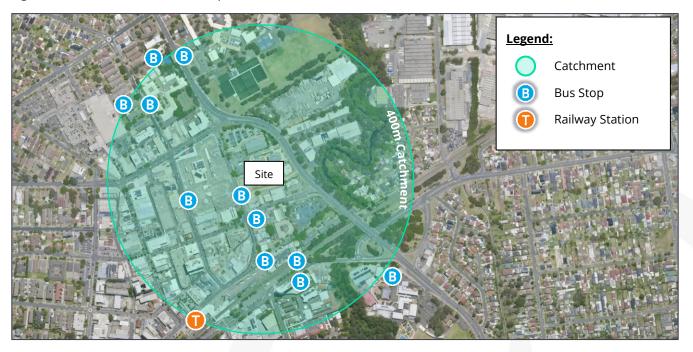
Traffic Control	Location		
	· Intersection(s) of:		
Traffic Signal	 The Horsley Drive, Nelson Street and Court Road 		
	o Court Road, Alan Street and Spencer Street		
Give-way / Stop	· Intersection(s) of:		
Control	o The Horsley Drive and Alan Street		
No Right Turn	· Intersection(s) of:		
No Right Turn	o The Horsley Drive and Alan Street		
School Zone	· Along part(s) of		
School Zone	o The Horsley Drive		



3.4 Public Transport Services

The local public transport services are illustrated in Figure 3-3.

Figure 3-3 Local Public Transport Locations



Source: Metromap (Modified by Genesis Traffic)

Train/Metro

The site is located approximately 400m to the north of Fairfield Railway Station providing connections to the Sydney Central Business District (CBD) via the Sydney Trains/Metro rail network.

<u>Bus</u>

Local bus service(s) within walking distance of the site are as follows (Table 3-3).

Table 3-3 Bus Services Provision

Bus Line	Bus Route
800	Blacktown to Fairfield via Pemulwuy
802	Liverpool to Parramatta via Guildford West
804	Liverpool to Parramatta via Fairfield West
808	Liverpool to Fairfield via Abbotsbury
812	Blacktown to Fairfield via Prospect Hwy
813	Bonnyrigg and Western Sydney Parklands to Fairfield
814	Fairfield to Smithfield and Wetherill Park Industrial Area (Loop Service)
817	Cabramatta to Fairfield via Edensor Park



904	Fairfield to Liverpool
905	Bankstown to Fairfield
906	Fairfield to Parramatta





4 Parking Assessment

4.1 Car Parking Requirement

Residential

Reference is made to the non-discretionary development standards in Chapter 2, Part 2 (In-fill Affordable Housing) of SEPP 2021, specifically section 19. The relevant car parking rates are provided in Table 4-1.

Table 4-1 Car Parking Rates from SEPP 2021

Land Use	Element	Minimum Parking Rates
	One-Bed	0.4 space(s) per unit
Affordable Housing	Two-Bed	0.5 space(s) per unit
	Three-Bed	1.0 space(s) per unit
	One-Bed	0.5 space(s) per unit
Non-affordable Housing	Two-Bed	1.0 space(s) per unit
	Three-Bed	1.5 space(s) per unit

<u>Retail</u>

The Fairfield City Centre DCP provides a car parking rate of 1 space per 25m². Applying the rate to the proposed retail floor area will generate a requirement of 42 spaces.

Overall Parking Provision

Application of the proposal using the above criteria would indicate the following requirement(s) in Table 4-2.

Table 4-2 Required Car Parking Spaces

Element		Unit/GFA	Requirement	Provision
	Studio / One-Bed	30 unit(s)	12	
Afferdable Herrine	Two-Bed	21 unit(s)	10.5	202
Affordable Housing	Three-Bed	6 unit(s)	6	
			Total: 28.5 spaces	
	Studio / One-Bed	92 unit(s)	46	382 spaces
Non-Affordable Housing	Two-Bed	170 unit(s)	170	
	Three-Bed	37 unit(s)	55.5	
			Total: 271.5 spaces	



Retail	42	52 spaces
Total	342 spaces	434 spaces

It is proposed to provide 434 parking spaces to comply with the above minimum criteria. These spaces will include:

- 382 x Resident
- 52 x Retail

4.2 Bicycle Parking and End of Trip Facility Requirement

The applicable bicycle parking rates (Table 4-3) are provided in Section 5.2.3 Bicycle Facilities in Fairfield City Centre DCP 2013.

Table 4-3 Bicycle Parking Rates

Development Type	Parking Rates	
Residential Units	1 space per 3 dwellings	
Commercial/Retail	1 space per 300m ²	

Application of the above DCP rates to the proposal indicates the following bicycle parking requirement(s) (Table 4-4).

Table 4-4 Required Bicycle Storage/Parking Spaces

Element	Unit/GFA	Requirement
Residential Units	356 units	119
Commercial/Retail	1,056.69m² GFA	4
Total		123 spaces

It is proposed to provide 135 bicycle spaces to comply with the DCP requirement.

4.3 Loading and Servicing Requirement & Arrangement

The DCP states the following in relation to waste collection vehicles/arrangement:

Service vehicles must be able to enter and exit the site in a forward direction. In order to reduce impact of people using primary streets for vehicle access, developments relying on vehicle access from the primary street must allow trucks to enter and exit in a forward direction (Note: The minimum size truck to service any development should be a medium rigid truck).

Furthermore, Fairfield Council has requested to provide a minimum of three (3) loading bays for a Small Rigid Vehicle (SRV) within the Ground Level car park.

Accordingly, the proposal will incorporate four (4) loading bays that suitable for:



- one (1) 12.5m Heavy Rigid Vehicle (HRV) located in the waste room
- three (3) 6.4m Small Rigid Vehicle (SRV): 1 located in the waste room and 2 located in the Ground Level car park

The HRV and SRV bays in the waste room have a sufficient headroom provision of 4.5m, satisfying the AS2890.2 headroom requirement. However, the two SRV bays in Ground Level car park have a headroom clearance of 2.7m, which falls short of the AS2890.2 requirement of 3.5m. Nonetheless, the assessment deems the provision of loading bays are sufficient and adequate to serve the development.

A comprehensive Loading Dock Management Plan (LDMP) can be documented prior to the occupation/construction stage, which outlines relevant measures to manage truck activities, including but not limited to:

- Implementing an online booking system to coordinate truck arrivals, vehicle types, heights, and frequencies
- Installing associated low headroom signages at the car park entry
- Outlining general operational rules for truck movements



5 Access and Circulation Design

5.1 Access, Design Assessment and Internal Circulation

The car parking access, circulation and arrangement are generally consistent with the approved scheme. Nonetheless, all critical vehicle movements in the proposed car parking facility have been assessed using Autoturn. Details of the assessment outcome, which demonstrate a satisfactory design provision, are provided in **Attachment 3**.





6 Traffic Assessment

6.1 Existing Traffic Conditions

Traffic surveys were commissioned as part of this assessment to record the AM peak and PM peak traffic flows at the intersections of:

- Court Road, Alan Street and Spencer Street
- The Horsley Drive, Nelson Road and Court Road

The traffic survey data is reproduced in **Attachment 4**.

The existing intersections' operation has been assessed using SIDRA traffic modelling program. SIDRA is a micro-analytical tool for individual and network intersection modelling based on collected traffic survey data. SIDRA provides a few performance indicators, as follows:

- Degree of Saturation the total usage of the intersection expressed as a factor of 1, with 1 representing 100% use/saturation.
- Average Delay the average delay encountered by all vehicles passing through the intersection.
- 95% Queue Length (Q95) is defined to be the queue length in metres that has only a 5% probability of being exceeded during the analysis period. It transforms the average delays into measurable distance units.
- Level of Service (LOS) this is a categorisation of average delay, intended for simple reference. The RMS adopts the following bands (Table 6-1)

Table 6-1 Intersection Performance – Levels of Service

Level of Service	Average Delay (s/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
A	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & Spare capacity
С	29 to 42	Satisfactory	Satisfactory but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity. At signals, incidents would cause excessive delays. Roundabouts require other control mode	At capacity and requires other mode of control



F	> 70	Extra capacity required	Extreme delay, major treatment required
---	------	-------------------------	--

An indication of prevailing traffic operations at these intersections is provided in the SIDRA assessment (Table 6-2).

Table 6-2 Existing Intersection Traffic Circumstance

Intersection	AM Peak		PM Peak		
	LOS	AVD	LOS	AVD	
The Horsley Drive, Nelson Road and Court Road	D	42.8s	D	52.2s	
Court Road and Alan Street	В	15.3s	В	16.4s	

Details of SIDRA results are reproduced in **Attachment 5**.

The assessment found the local road network operating with ample spare capacity under existing traffic demand (including the existing site's traffic movements).

6.2 Approved Outcome Traffic Generation & Impact

The Traffic Assessment¹ that accompanied the approved development scheme envisaged the following traffic generation outcome associated with the then standard apartment units and retail floor space:

Table 6-3 Approved Traffic Generation Outcome

		AM Peak		PM	Peak
	Total	ln	Out	In	Out
High Density Residential	84 vtph	14	70	70	14
Retail	80 vtph	40	40	40	40
Total		54	110	110	54

On the basis of the above, the Traffic Assessment found the traffic generation acceptable which underpinned the subsequent development consent.

¹ McLaren Traffic Engineering, Traffic Impact Assessment, September 2014



6.3 Development Traffic Generation

Residential

Since the original assessment, the Guide to Traffic Generating Development has been updated with the Technical Direction 2013/04a, providing revised trip generation rates for high-density residential apartment blocks in proximity of railway stations. The relevant trip rates are as follows:

- 0.19 vtph per unit during the morning peak hour
- 0.15 vtph per unit during the evening peak hour

However, it is likely that the eventual traffic generation outcome of the now proposed affordable housing will be lower than that of a high-density residential development due to the lower tendency of occupants to own private vehicles. This is also reflected in the affordable housing's lower parking requirement when compared with that of a standard high-density apartment.

Nevertheless, applying these conservative trip rates to the proposed units would indicate a peak-hour traffic generation outcome in Table 6-4.

Table 6-4 Peak Hour Traffic Generation

Period	Total	In	Out
AM peak	68 vtph	8 vtph	60 vtph
PM peak	54 vtph	48 vtph	6 vtph

Retail

The Guide to Traffic Generating Development provides an evening peak traffic generation rate of 5.6 vtph per 100m² GFA for retail floor space as per the approved outcome. Application of this rate to the proposed retail floor space would indicate a peak hour traffic generation outcome of 60 vtph.

6.4 Overall Traffic Generation and Distribution

The expected overall net traffic generation outcome is tabulated in Table 6-5 as follows:

Table 6-5 Net Peak Hour Traffic Generation

	AM Peak (vtph)		PM Peak (vtph)		
<u>Period</u>	<u>ln</u>	<u>Out</u>	<u>ln</u>	<u>Out</u>	
Residential	8	60	48	6	
Retail	30	30	30	30	
Net Traffic	38	90	78	36	



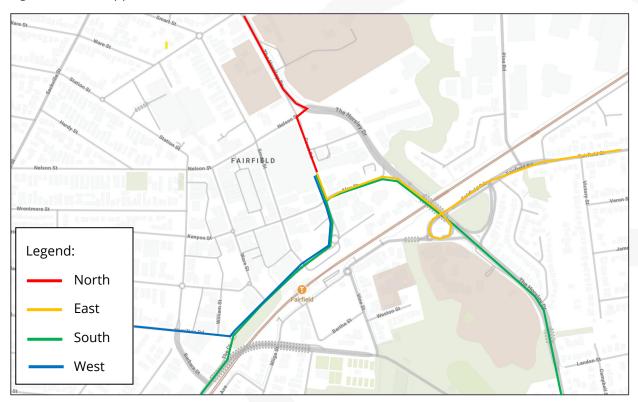
According to the 2016 Journey to Work Data and the demographic of the surrounding land uses, the proportion of each direction is tabulated Table 6-6.

Table 6-6 Proportion of Inbound and Outbound Traffic

Direction		Proportion		
	<u>Residential</u>	Commercial/Retail		
North	35%	25%		
East	15%	25%		
South	15%	25%		
West	35%	25%		

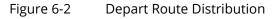
The nominal route choices from/to each direction are illustrated in Figure 6-1.

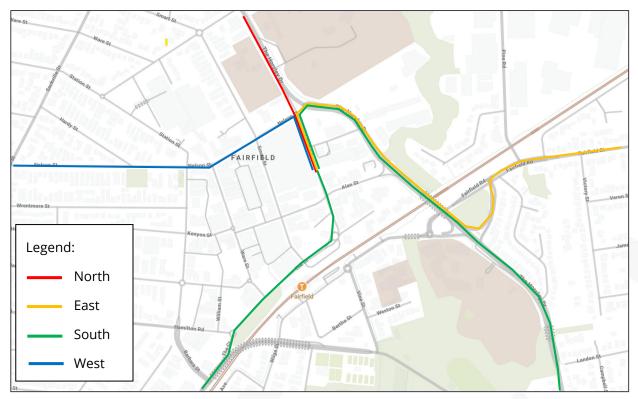
Figure 6-1 Approach Route Distribution



Source: Mecone (modified by Genesis Traffic)





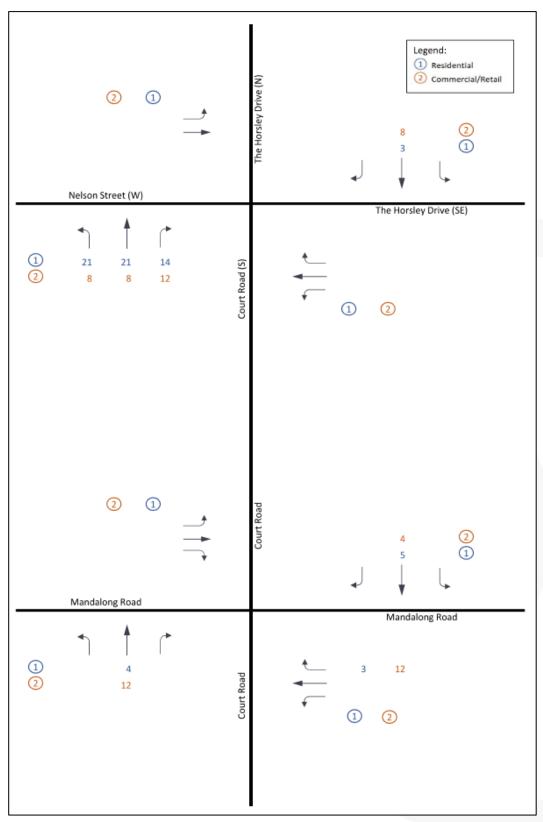


Source: Mecone (modified by Genesis Traffic)

On this basis, once the directional splits are taken into consideration in this assessment, the resulting traffic generation from each direction is illustrated in Figure 6-3 and Figure 6-4.



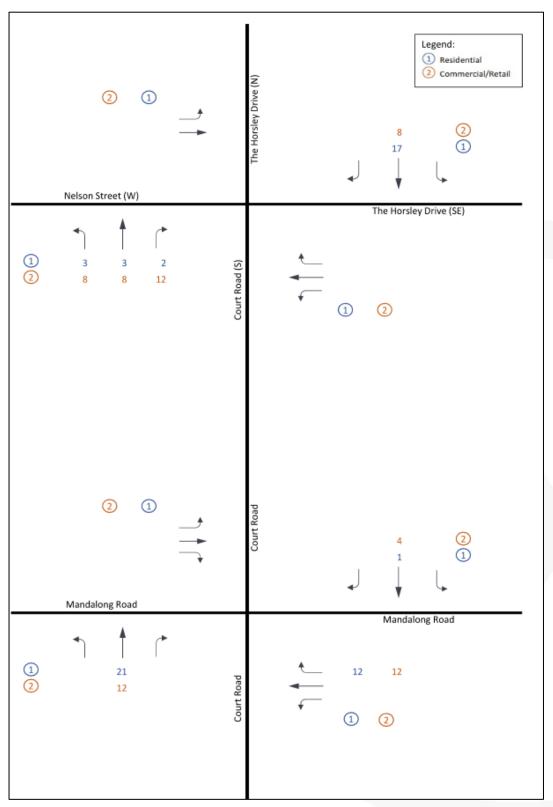
Figure 6-3 Inbound and Outbound Traffic during AM Peak



Source: Genesis Traffic



Figure 6-4 Inbound and Outbound Traffic during PM Peak



Source: Genesis Traffic



The projected development traffic are then added onto the surveyed background traffic and reanalysed using SIDRA. The assessment considers the existing state, the existing state with the subject development to provide an objective comparison in terms of traffic generation impact on the network. The assessed model outcome is summarised in Table 6-7.

Table 6-7 Existing and Post-Development SIDRA Assessment Outcome

Intersection	AM Peak		PM Peak	
	LOS	AVD	LOS	AVD
Pre-development				
The Horsley Drive, Nelson Road and Court Road	D	42.8s	D	52.2s
Court Road and Alan Street	В	15.3s	В	16.4s
Post Development				
The Horsley Drive, Nelson Road and Court Road	D	47.7s	D	55.1s
Court Road and Alan Street	В	15.4s	В	16.9s

The SIDRA output is reproduced in **Attachment 5**.

The assessment found the existing road network operate with ample spare capacity and the road network levels of service will be maintained following the addition of the subject development.

On this basis, the assessment concludes that the development would not adversely impact the existing road network.



7 Conclusion

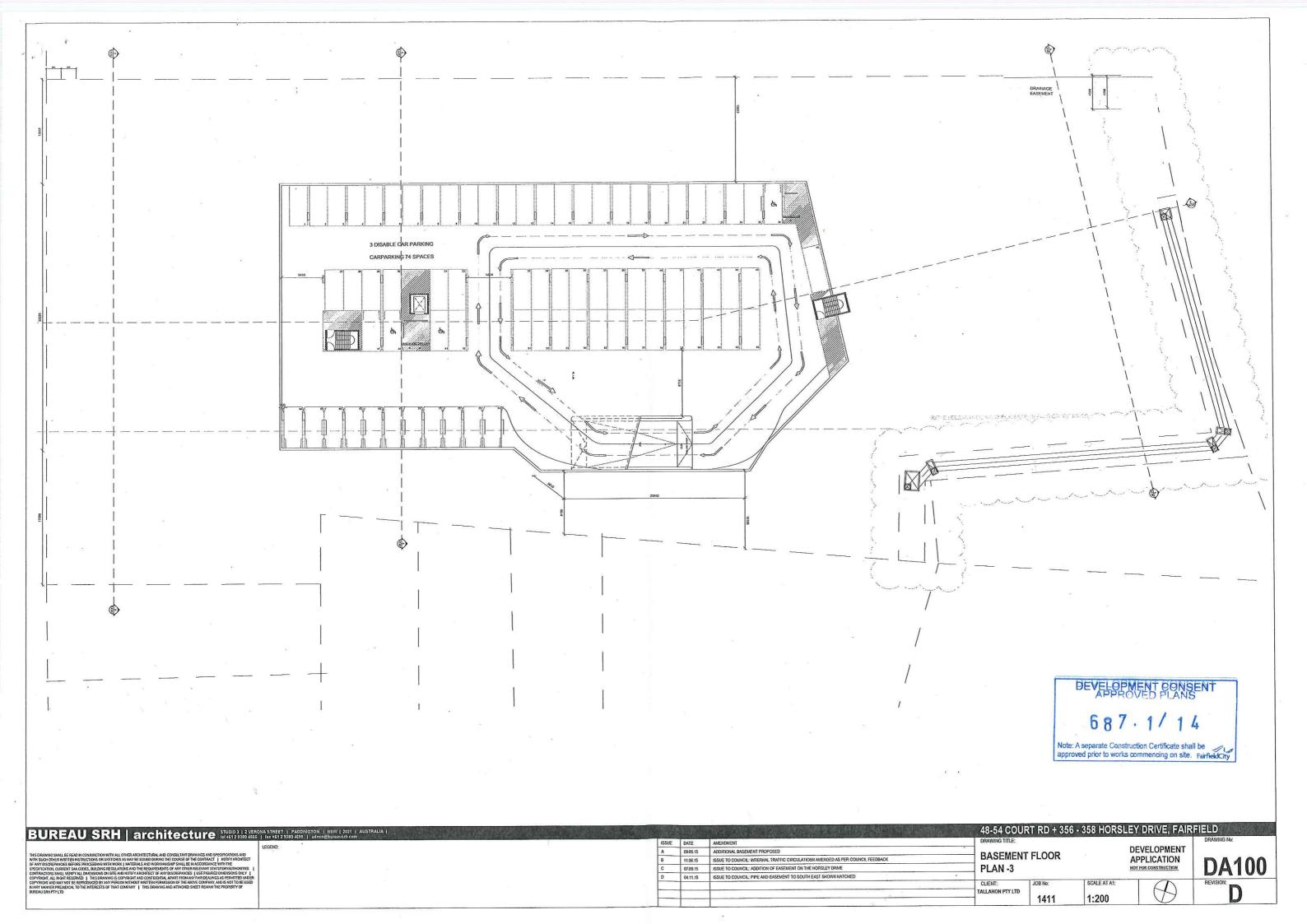
The traffic and parking assessment undertaken for the Proposed Mixed-Use Development at 46-54 Court Road, Fairfield has concluded that:

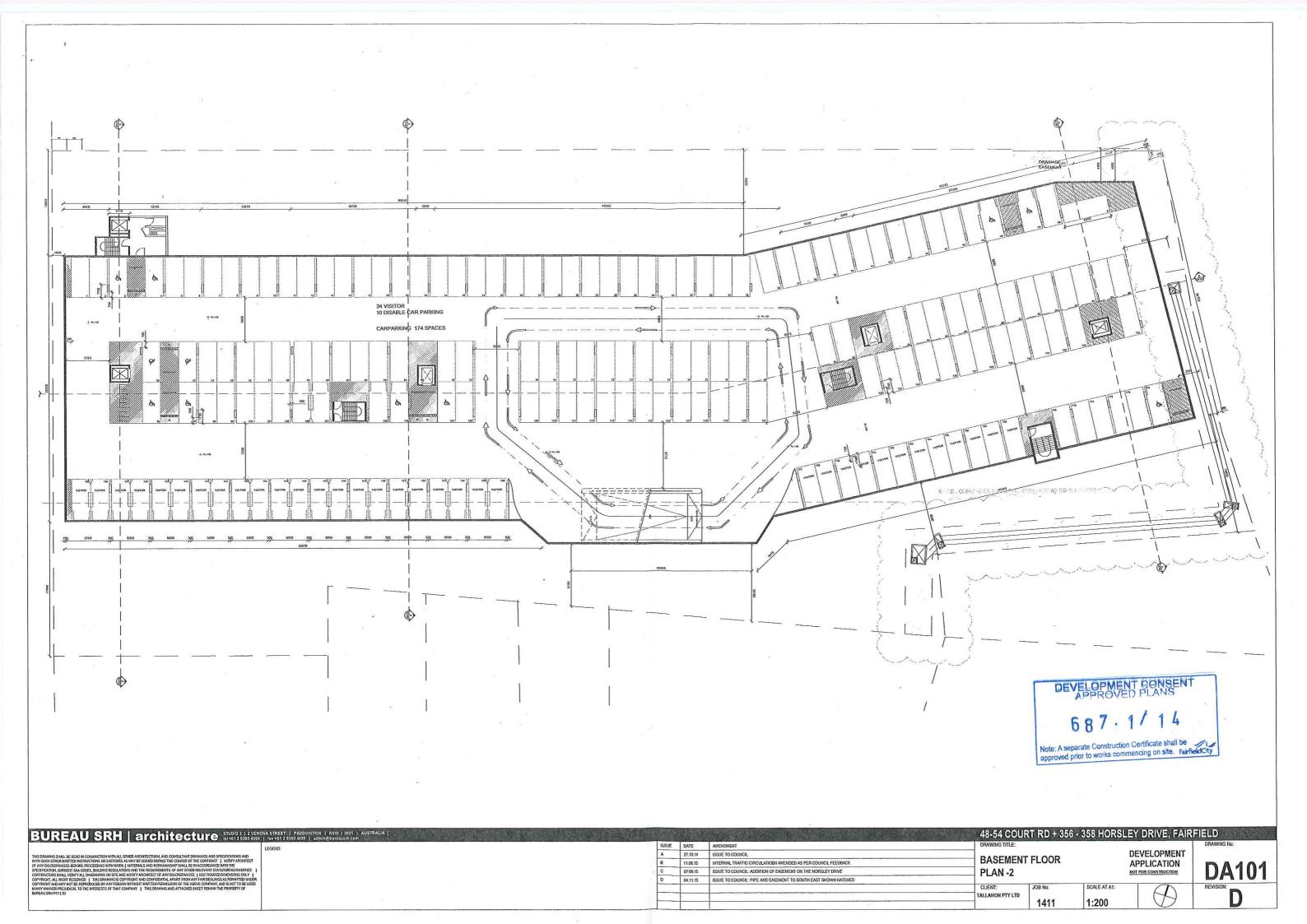
- the traffic generation of the proposed development will not present any adverse traffic implications
- the proposed parking provision will comply with the SEPP (Housing) 2021 and DCP criteria, and will adequately serve the development
- the proposed access, internal circulation and parking arrangements will be appropriate to AS design criteria

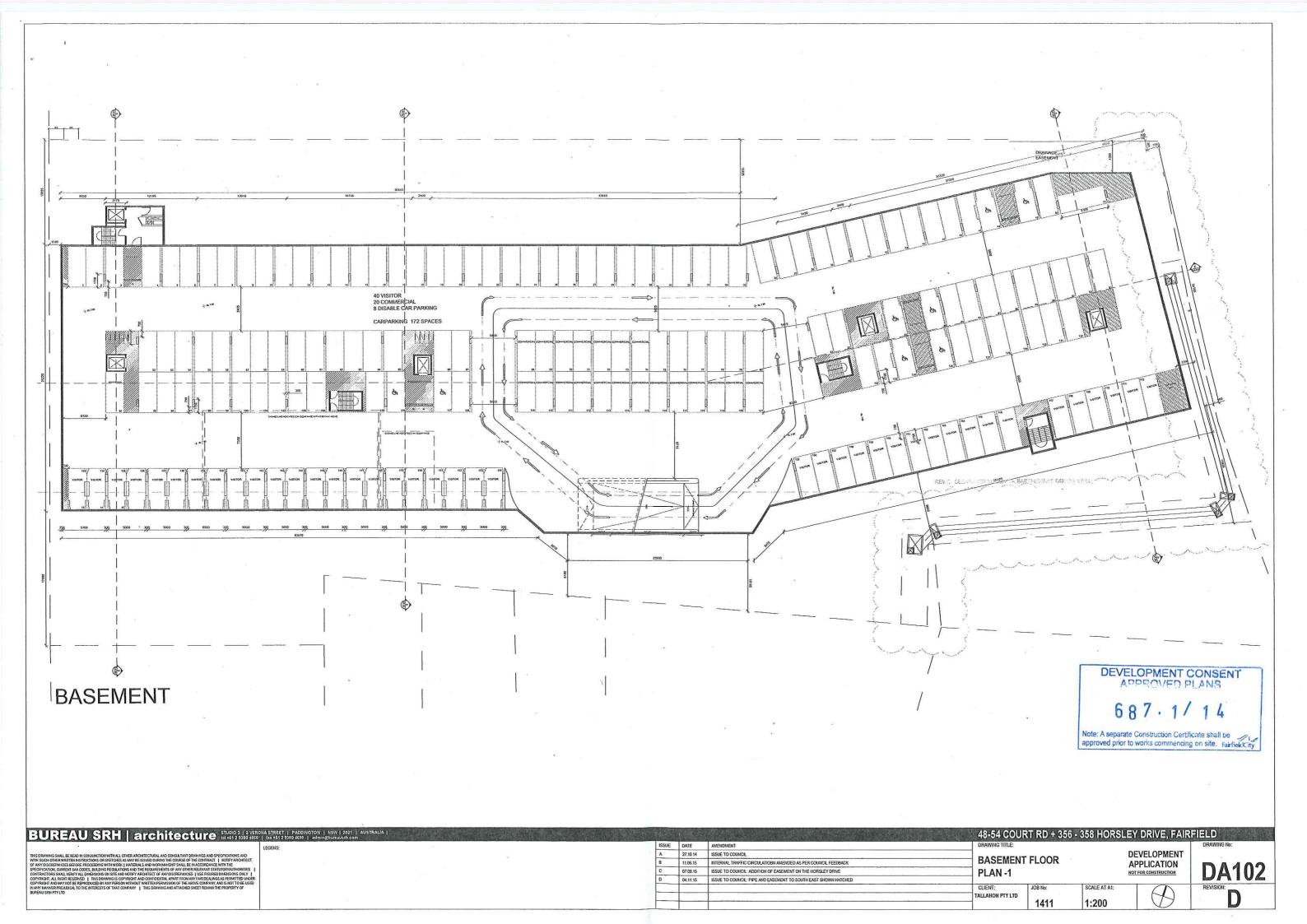


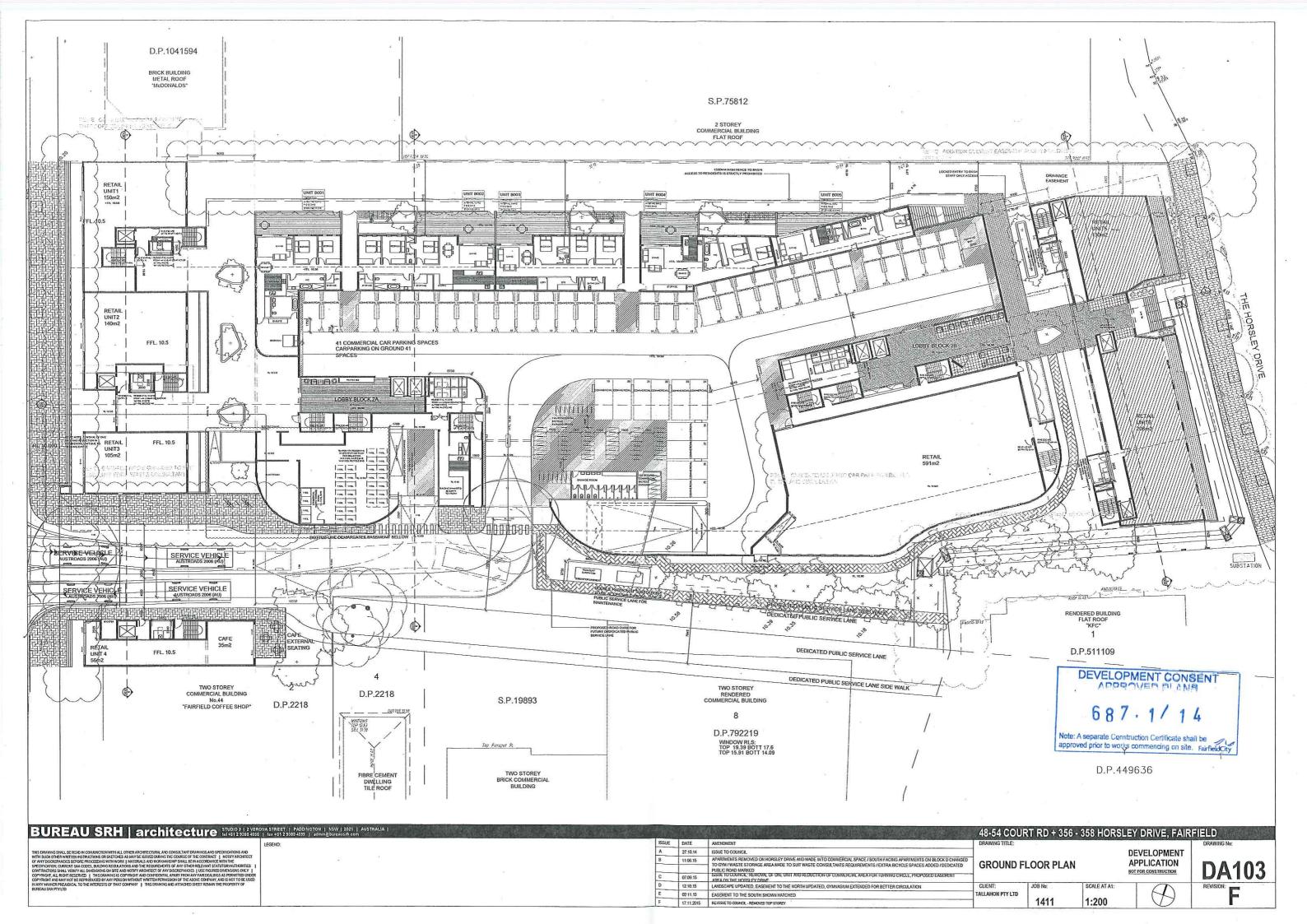
Attachment 1

Approved DA Plan





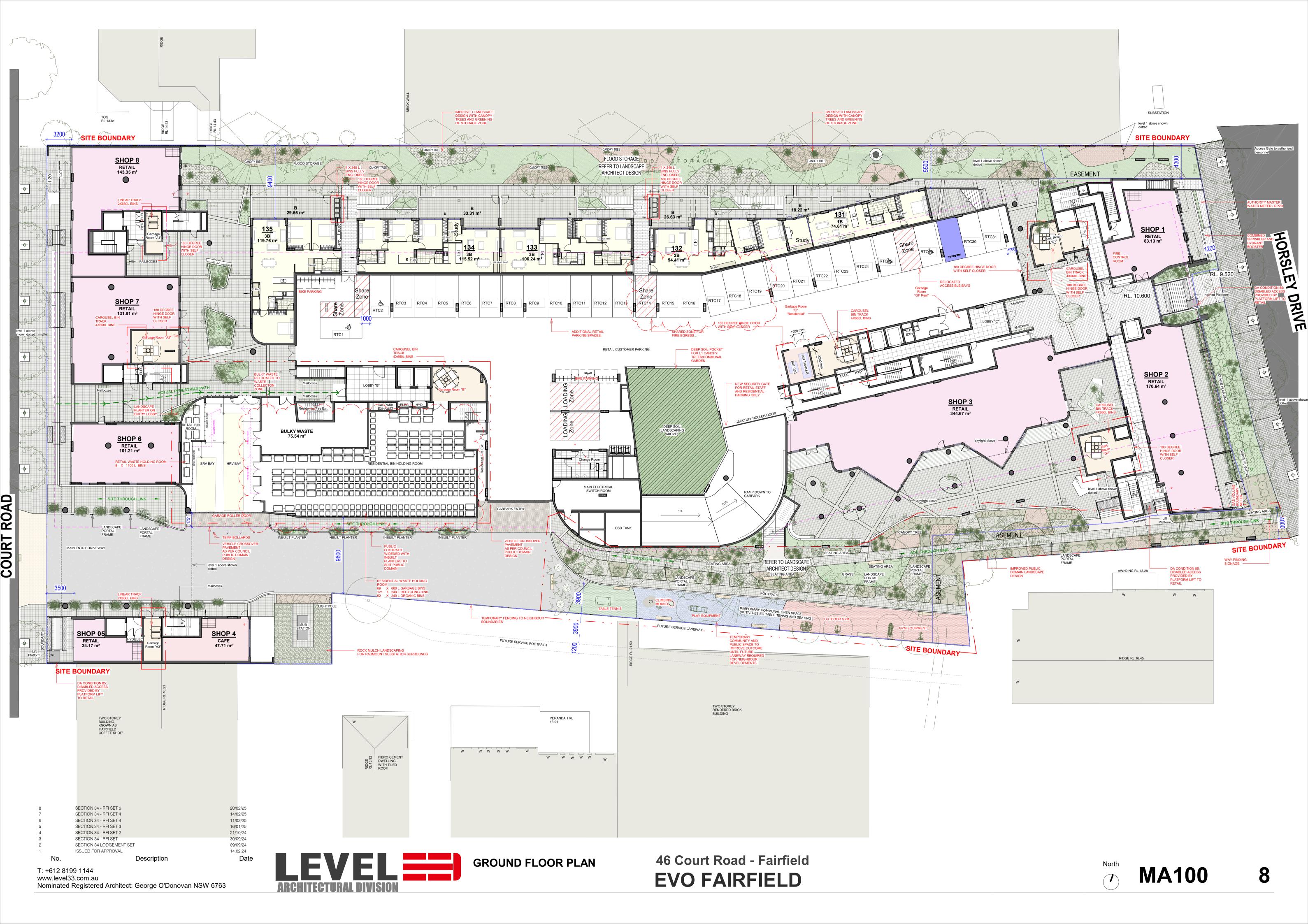


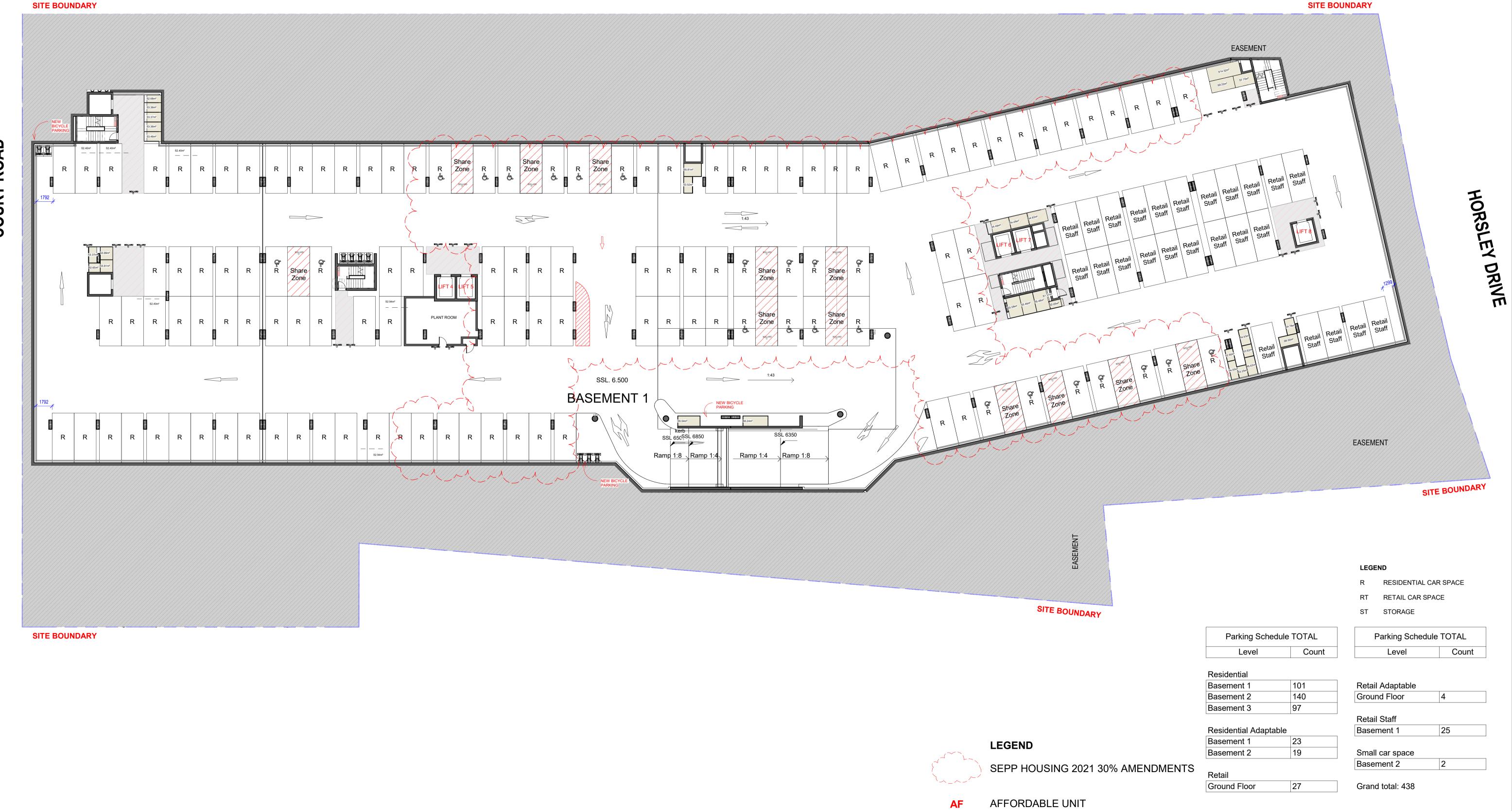




Attachment 2

Proposed Plan





SECTION 34 - RFI SET 3 SECTION 34 - RFI SET 2 SECTION 34 - RFI SET SECTION 34 LODGEMENT SET ISSUED FOR APPROVAL Description

LEVEL

16/01/25 21/10/24 30/09/24

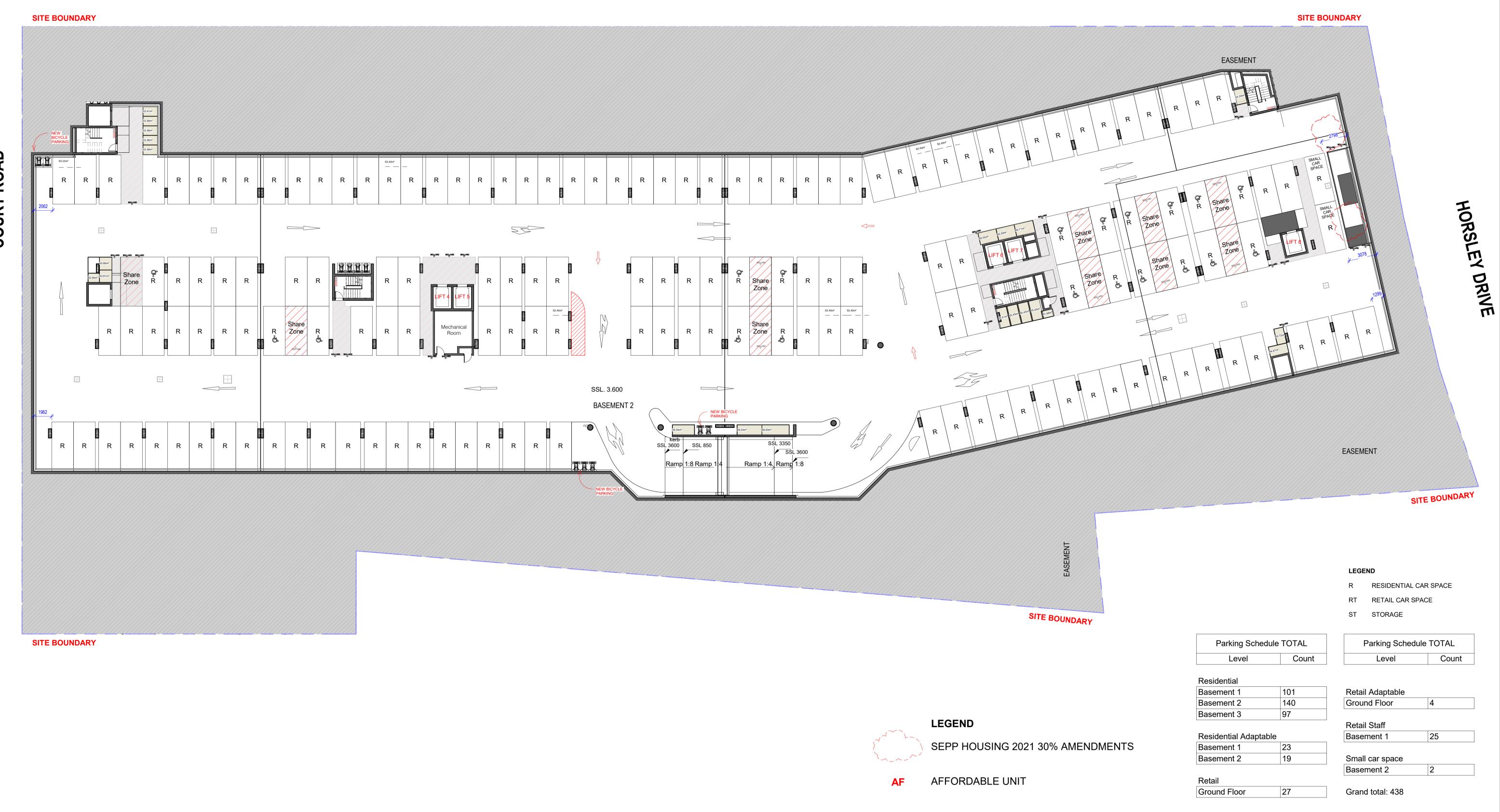
09/09/24

14.02.24

BASEMENT 1 PLAN

46 Court Road - Fairfield **EVO FAIRFIELD**

MA099



SECTION 34 - RFI SET 3 SECTION 34 - RFI SET 2 SECTION 34 - RFI SET SECTION 34 LODGEMENT SET ISSUED FOR APPROVAL Description T: +612 8199 1144

LEVEL

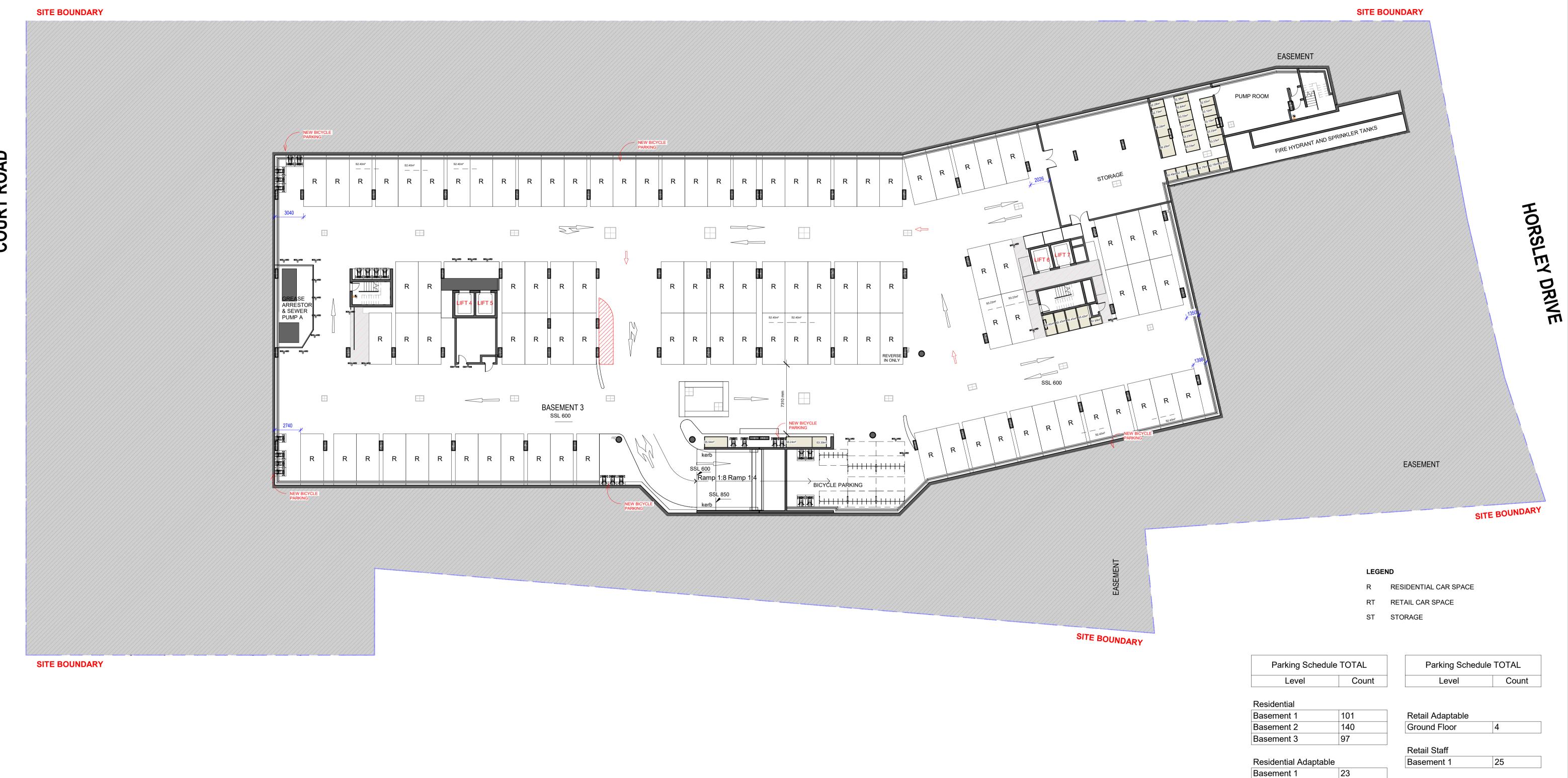
16/01/25 21/10/24 30/09/24

09/09/24

BASEMENT 2 PLAN

46 Court Road - Fairfield **EVO FAIRFIELD**

MA098



SECTION 34 - RFI SET 3 SECTION 34 - RFI SET 2 SECTION 34 - RFI SET SECTION 34 LODGEMENT SET ISSUED FOR APPROVAL Description

T: +612 8199 1144

www.level33.com.au

16/01/25 21/10/24 30/09/24 09/09/24 14.02.24

LEVEL **ARCHITECTURAL DIVISION**

BASEMENT 3 PLAN

46 Court Road - Fairfield **EVO FAIRFIELD**

19

27

Basement 2

Ground Floor

Retail

MA097

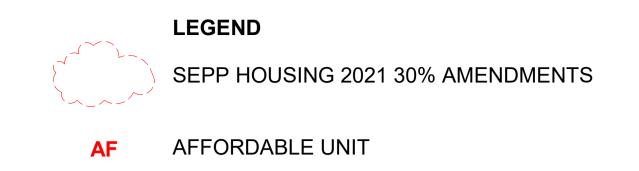
Small car space

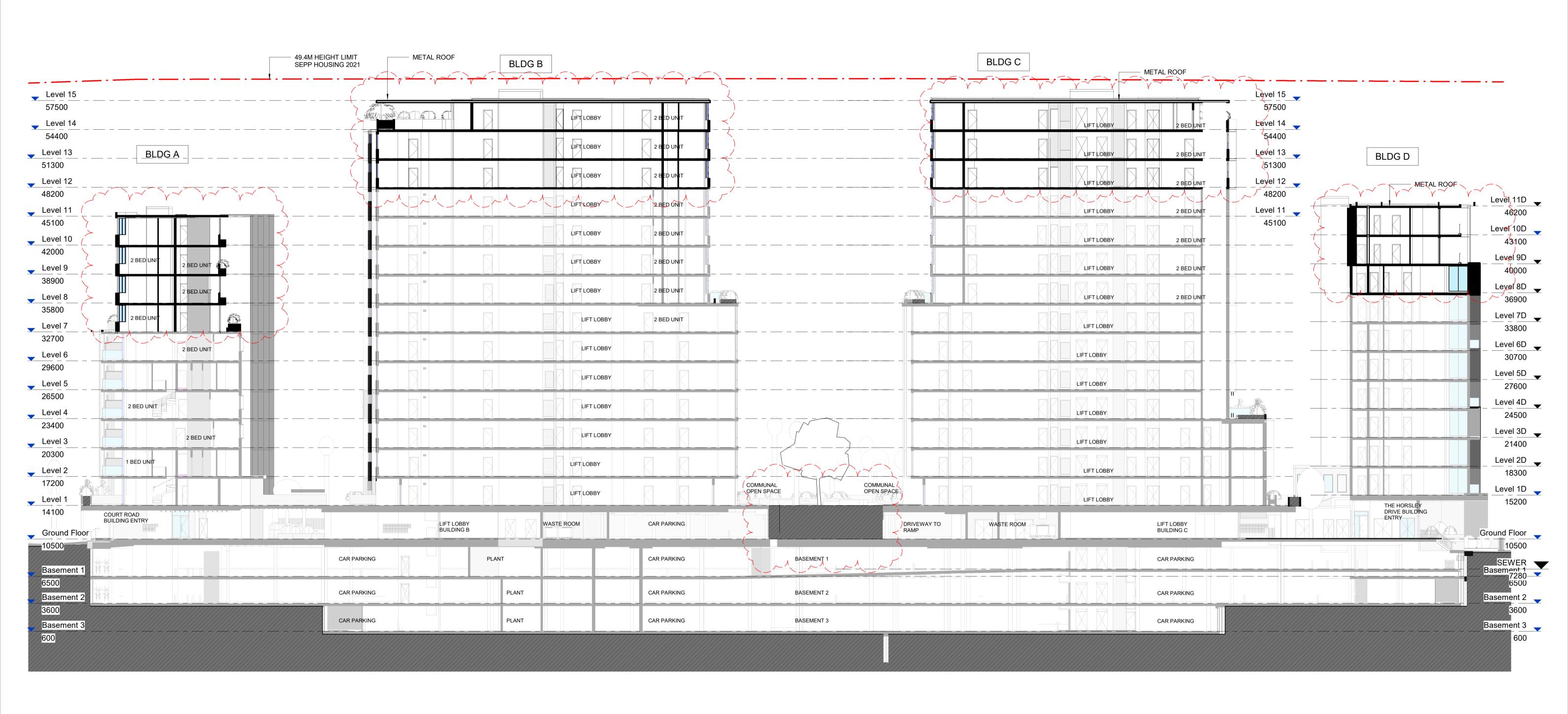
Grand total: 438

Basement 2

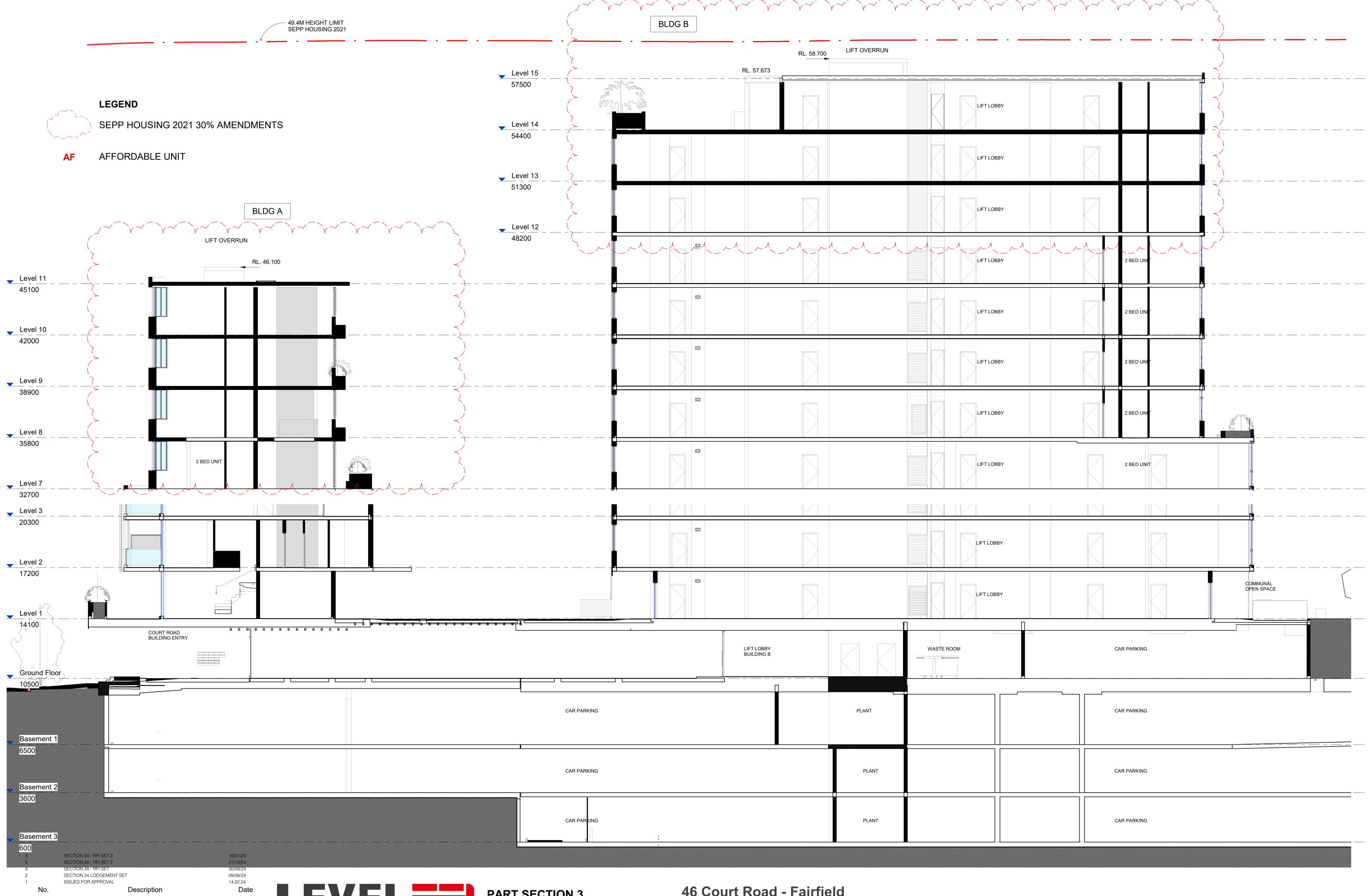
5

2





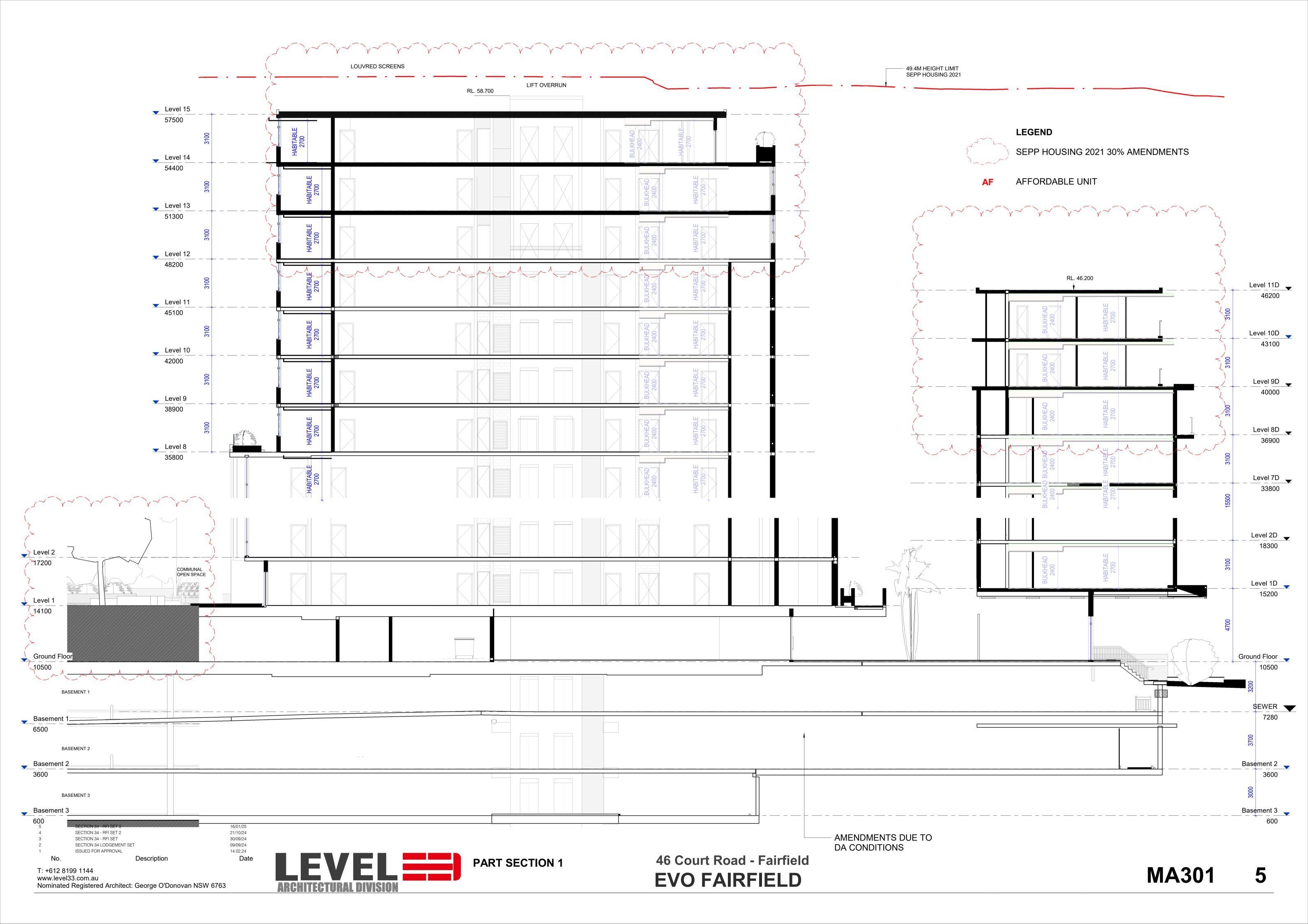




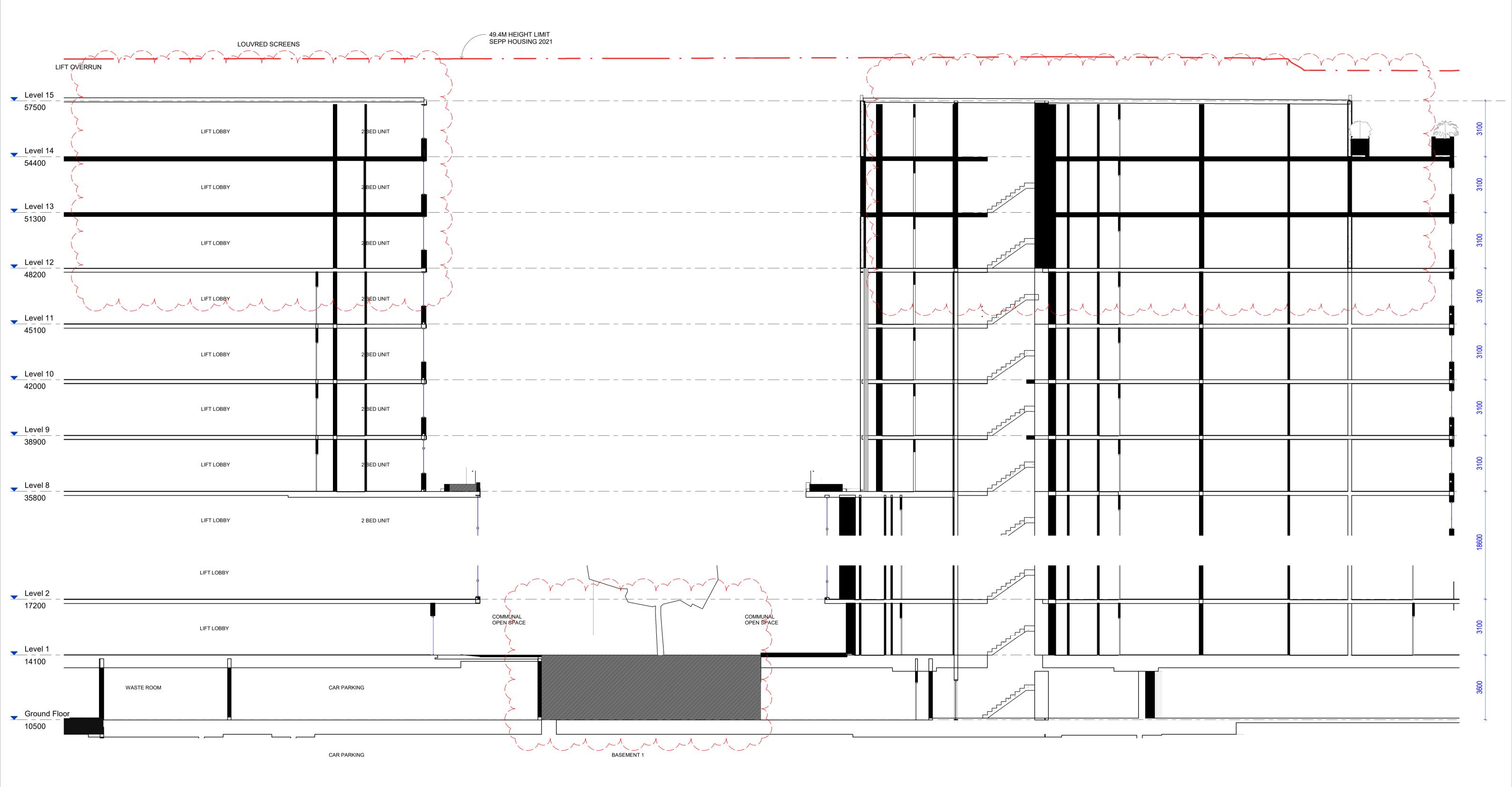
T: +612 8199 1144

www.level33.com.au

Nominated Registered Architect: George O'Donovan NSW 6763



BLDG B



www.level33.com.au Nominated Registered Architect: George O'Donovan NSW 6763





Attachment 3

Turning Path Assessment

PROJECT

55 COURT ROAD, FAIRFIELD

TITLE

COMPLIANCE ASSESSMENT

GROUND FLOOR





DESIGNED BY L.NG REVIEWED BY B.LO

DRAWING REFERENCE (SOURCE):
G:\2024\24015 - 55 COURT ROAD,
FAIRFIELD (AMENDED
DA)\DRAWINGS\20250220

ISSUE DATE

20 February 2025 01 OF 11

DRAWING REF NO. 24015-V1.10-SP

LEGENDS/NOTES

SHEET NO.

- MAXIMUM CHANGE IN GRADE FOR CARS SHOULD BE 1:8 OVER 2m
- MINIMUM HEIGHT CLEARANCE OF 2.2m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE CIRCULATION AISLES AND PARKING SPACES
- MINIMUM HEIGHT CLEARANCE OF 2.5m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE ACCESSIBLE PARKING SPACES
- MINIMUM HEIGHT CLEARANCE
 OF 4.5m (TO SERVICES AND
 STRUCTURE) SHOULD BE
 PROVIDED ABOVE SERVICE
 VEHICLE ACCESS ROADWAYS
 AND LOADING DOCKS
 HEIGHT CLEARANCE ABOVE A
- SAG CHANGE IN GRADES
 SHOULD BE MEASURED IN
 ACCORDANCE WITH FIGURE 5.3
 AS2890.1:2004

PROJECT

55 COURT ROAD, FAIRFIELD

TITLE

COMPLIANCE ASSESSMENT

BASEMENT 1



DESIGNED BY REVIEWED BY
L.NG B.LO

DRAWING REFERENCE (SOURCE):
G:\2024\24015 - 55 COURT ROAD,
FAIRFIELD (AMENDED
DA)\DRAWINGS\20250220

 ISSUE DATE
 20 F US TO STATE T

LEGENDS/NOTES

- MAXIMUM CHANGE IN GRADE FOR CARS SHOULD BE 1:8 OVER 2m
- MINIMUM HEIGHT CLEARANCE OF 2.2m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE CIRCULATION AISLES AND PARKING SPACES
- MINIMUM HEIGHT CLEARANCE OF 2.5m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE ACCESSIBLE PARKING SPACES
- MINIMUM HEIGHT CLEARANCE OF 4.5m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE SERVICE VEHICLE ACCESS ROADWAYS AND LOADING DOCKS
- HEIGHT CLEARANCE ABOVE A SAG CHANGE IN GRADES SHOULD BE MEASURED IN ACCORDANCE WITH FIGURE 5.3 AS2890.1:2004

PROJECT

55 COURT ROAD, FAIRFIELD

COMPLIANCE ASSESSMENT

BASEMENT 2



DESIGNED BY L.NG

REVIEWED BY B.LO

DRAWING REFERENCE (SOURCE): G:\2024\24015 - 55 COURT ROAD, FAIRFIELD (AMENDED DA)\DRAWINGS\20250220

ISSUE DATE

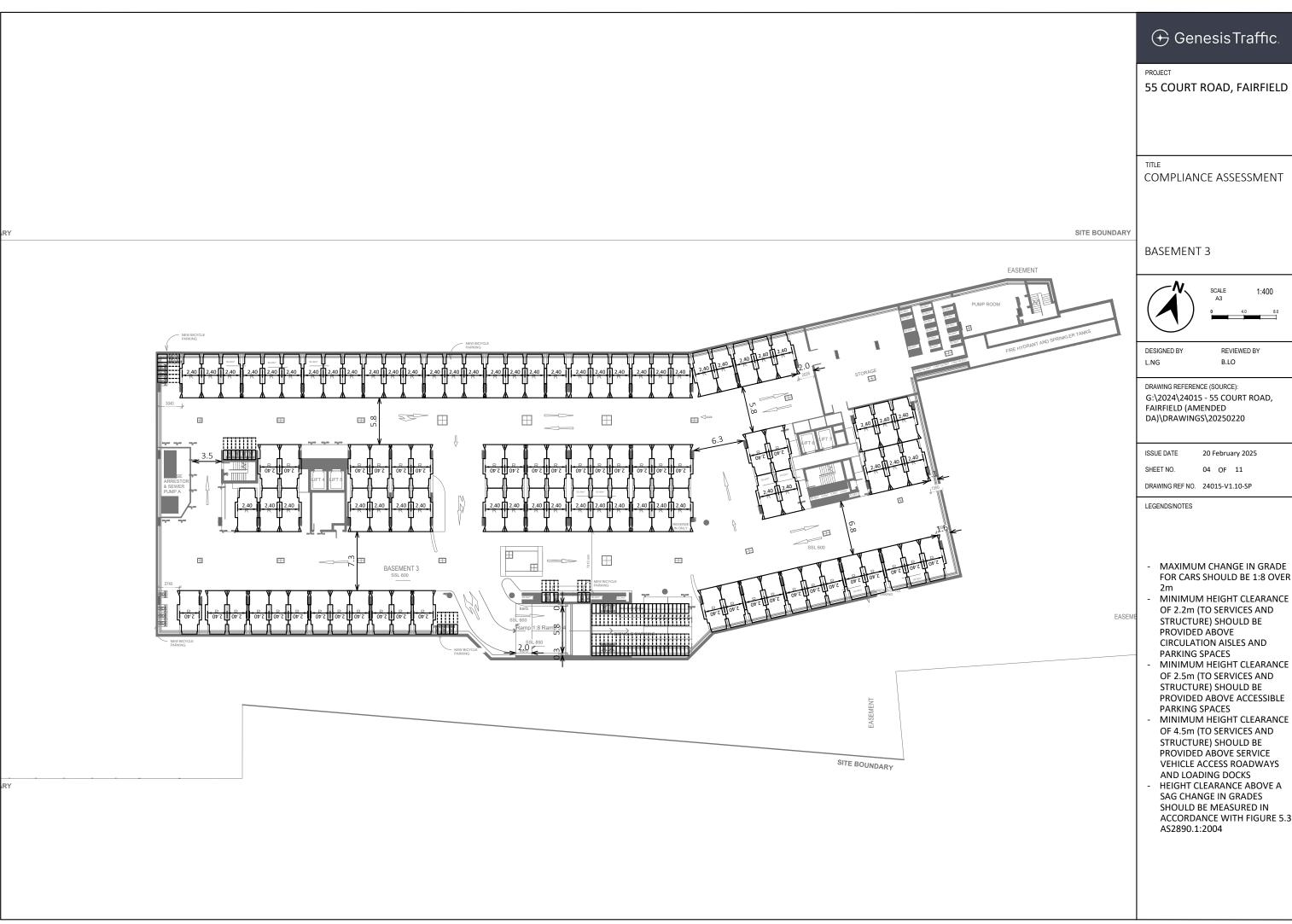
20 February 2025 03 OF 11

DRAWING REF NO. 24015-V1.10-SP

LEGENDS/NOTES

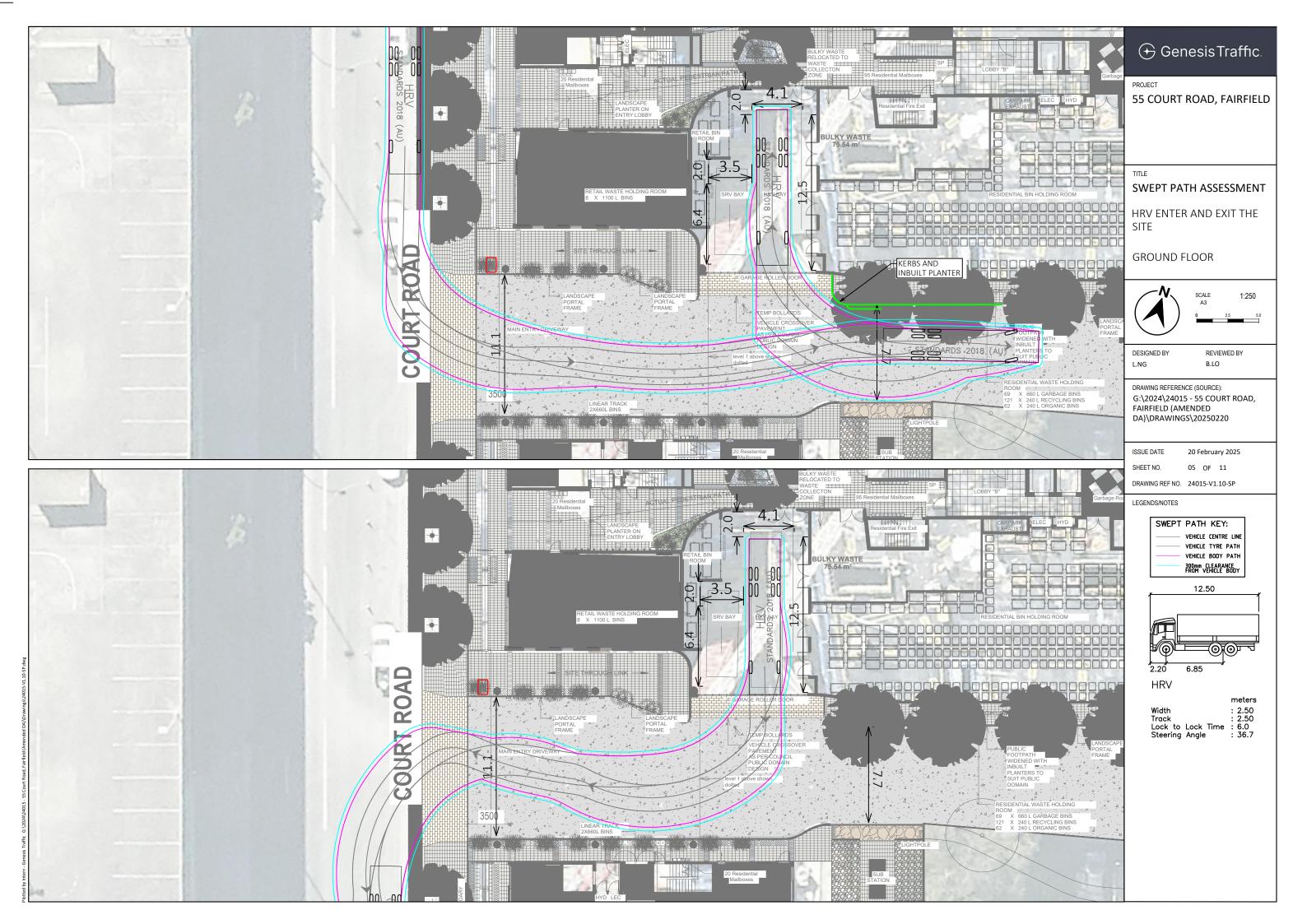
SHEET NO.

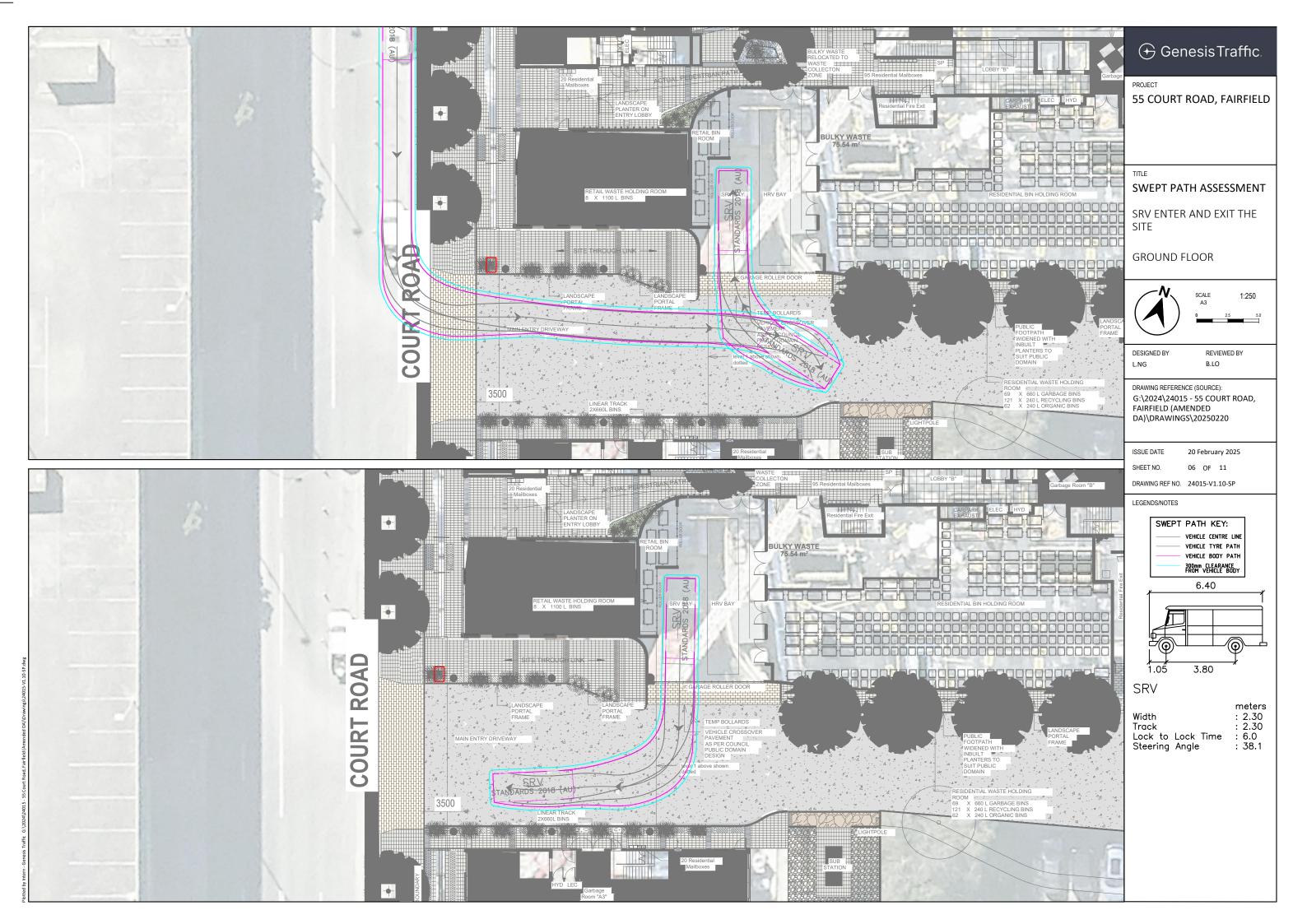
- MAXIMUM CHANGE IN GRADE FOR CARS SHOULD BE 1:8 OVER
- MINIMUM HEIGHT CLEARANCE OF 2.2m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE CIRCULATION AISLES AND PARKING SPACES
- MINIMUM HEIGHT CLEARANCE OF 2.5m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE ACCESSIBLE PARKING SPACES
- MINIMUM HEIGHT CLEARANCE OF 4.5m (TO SERVICES AND STRUCTURE) SHOULD BE PROVIDED ABOVE SERVICE VEHICLE ACCESS ROADWAYS AND LOADING DOCKS
- HEIGHT CLEARANCE ABOVE A SAG CHANGE IN GRADES SHOULD BE MEASURED IN ACCORDANCE WITH FIGURE 5.3 AS2890.1:2004

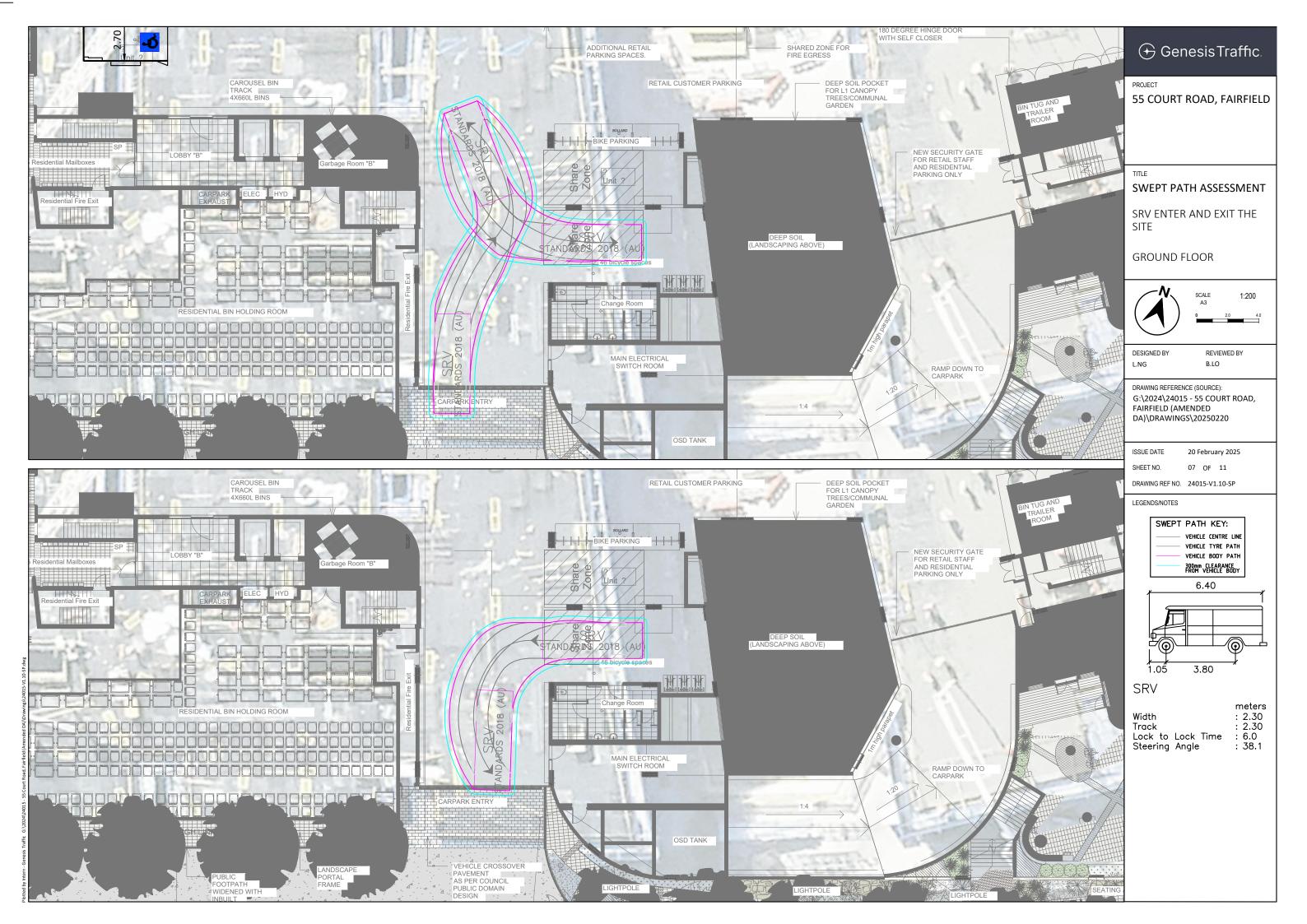


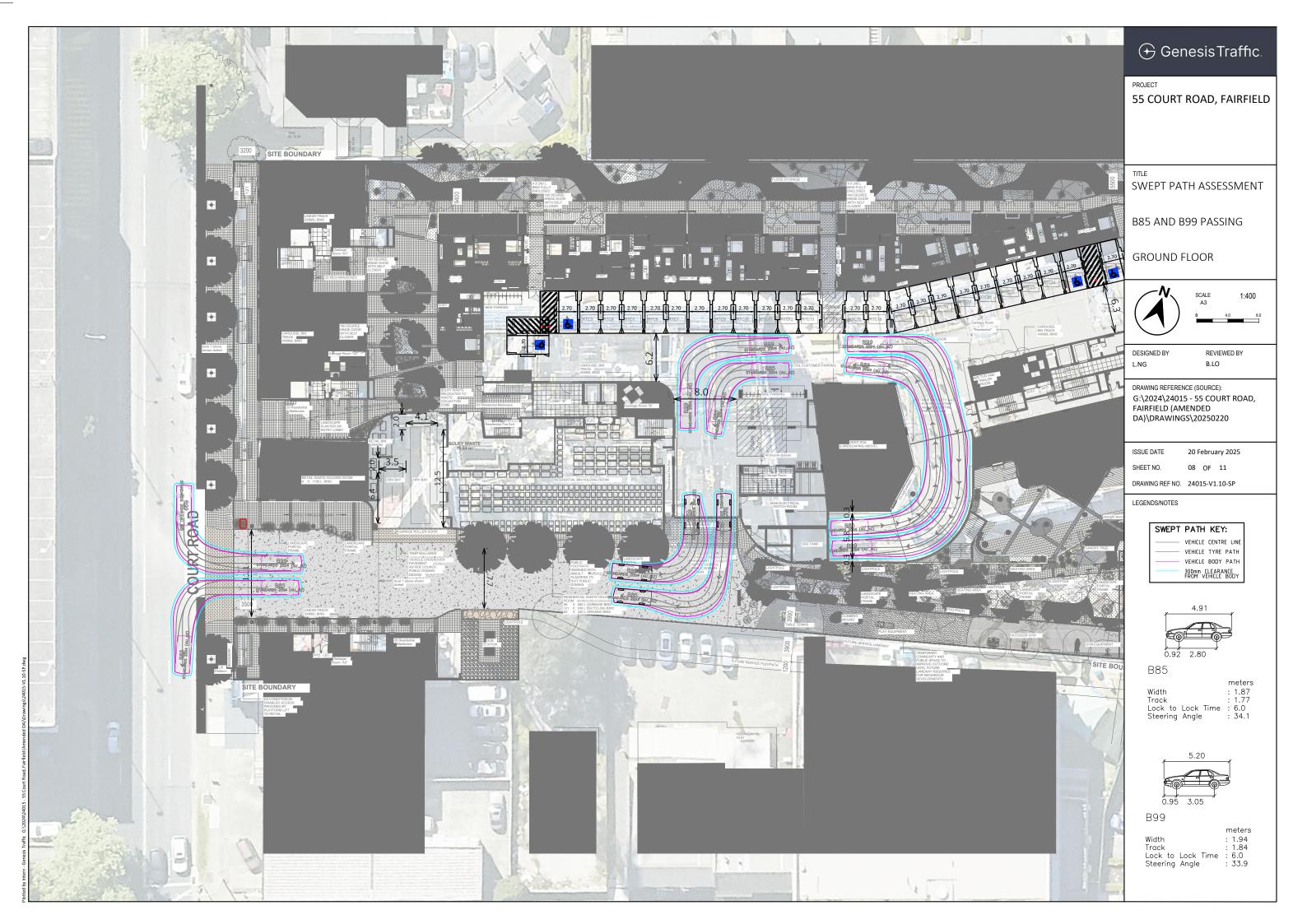


- FOR CARS SHOULD BE 1:8 OVER
- OF 2.5m (TO SERVICES AND PROVIDED ABOVE ACCESSIBLE
- OF 4.5m (TO SERVICES AND VEHICLE ACCESS ROADWAYS
- ACCORDANCE WITH FIGURE 5.3









PROJECT

55 COURT ROAD, FAIRFIELD

TITLE

SWEPT PATH ASSESSMENT

B85 AND B99 PASSING

BASEMENT 1





DESIGNED BY L.NG REVIEWED BY B.LO

20 February 2025

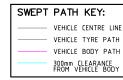
DRAWING REFERENCE (SOURCE):
G:\2024\24015 - 55 COURT ROAD,
FAIRFIELD (AMENDED
DA)\DRAWINGS\20250220

ISSUE DATE SHEET NO.

09 OF 11

DRAWING REF NO. 24015-V1.10-SP

LEGENDS/NOTES





B85

Width : 1.87
Track : 1.77
Lock to Lock Time : 6.0
Steering Angle : 34.1



B9

Width : 1.94
Track : 1.84
Lock to Lock Time : 6.0
Steering Angle : 33.9

PROJECT

55 COURT ROAD, FAIRFIELD

TITLE

SWEPT PATH ASSESSMENT

B85 AND B99 PASSING

BASEMENT 2



DESIGNED BY REVIEWED BY
L.NG B.LO

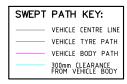
DRAWING REFERENCE (SOURCE):
G:\2024\24015 - 55 COURT ROAD,
FAIRFIELD (AMENDED
DA)\DRAWINGS\20250220

 ISSUE DATE
 20 February 2025

 SHEET NO.
 10 OF 11

 DRAWING REF NO.
 24015-V1.10-SP

LEGENDS/NOTES





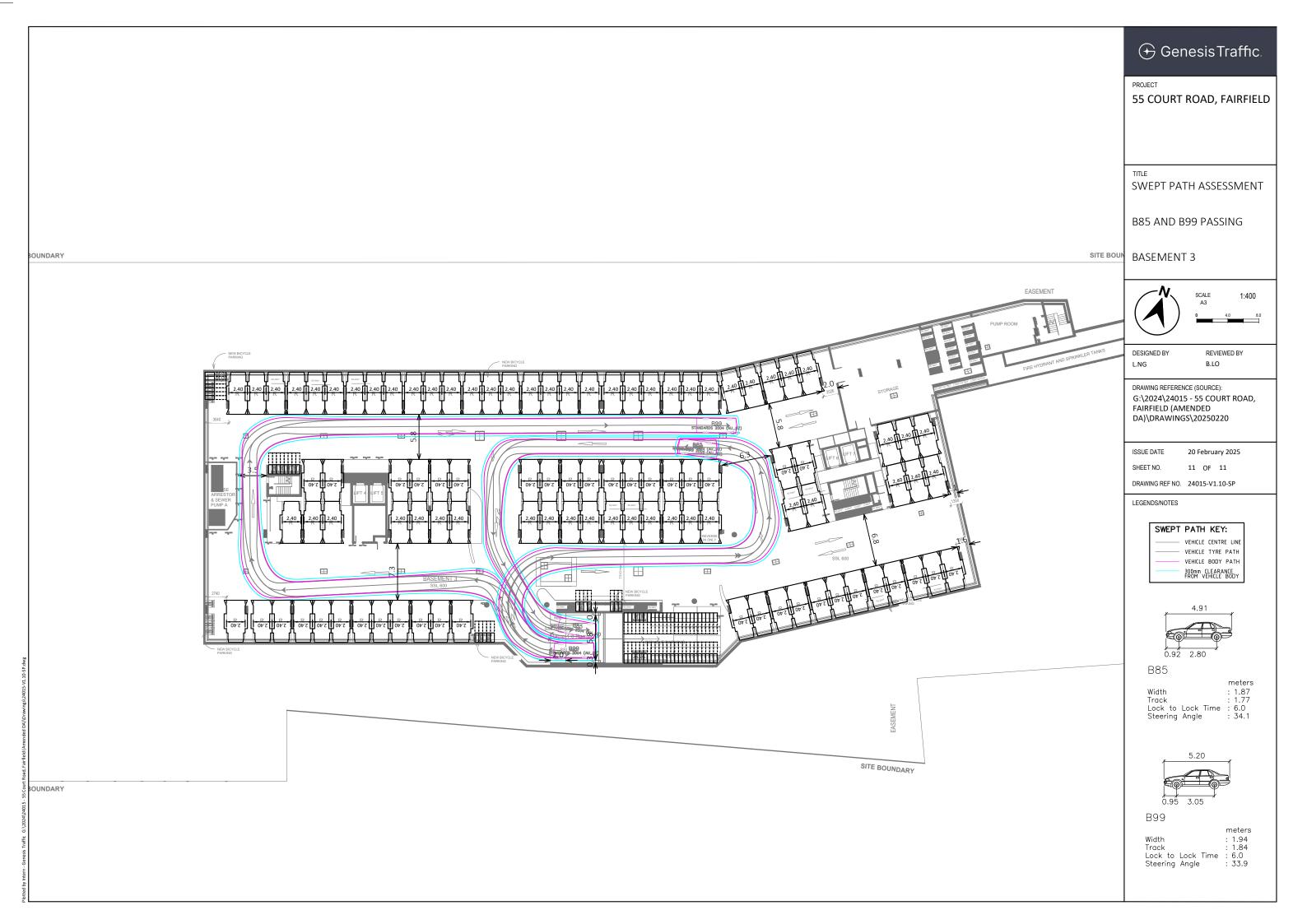
B85

width : 1.87
Track : 1.77
Lock to Lock Time : 6.0
Steering Angle : 34.1



B99

Width : 1.94
Track : 1.84
Lock to Lock Time : 6.0
Steering Angle : 33.9





Attachment 4

Traffic Survey



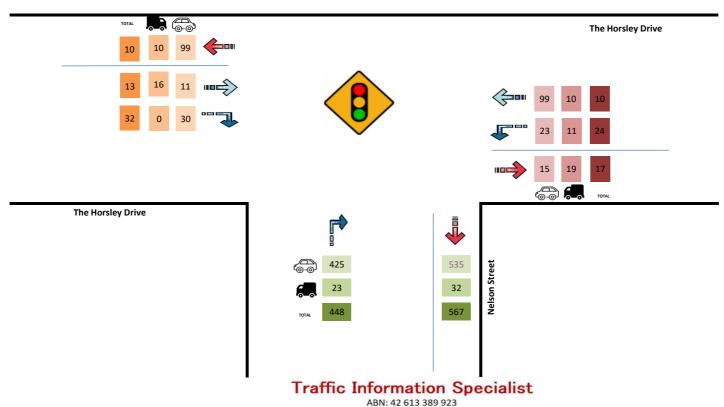
Location	-	Duration	7:00 - 9:00	
	The Horsley Drive		16:00 - 18:00	
	Nelson Street			
	The Horsley Drive	Day/Date	Wednesday, 21 February 2024	
Suburb	FAIRFIELD	Weather	-	

DATA SELECTION

Select Time: PEAK ▼

TI	ME RANG	GE .
PEAK	-	AM
	PEAK	
8:00	-	9:00





ABN: 42 613 389 923 Email info@tistraffic.com.au



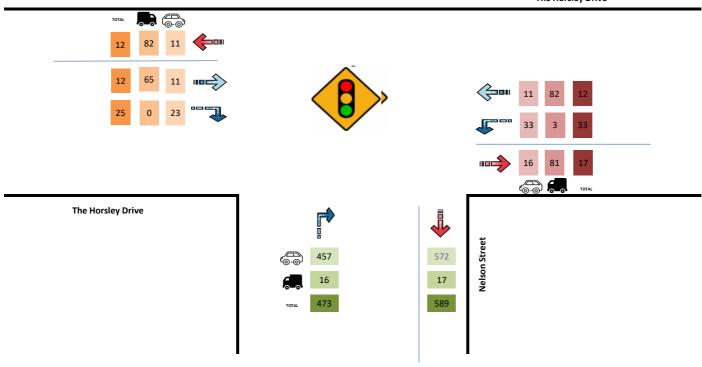
Location _	-	Duration	7:00 - 9:00	
	The Horsley Drive		16:00 - 18:00	
_	Nelson Street		-	
_	The Horsley Drive	Day/Date	Wednesday, 21 February 2024	
Suburb	FAIRFIELD	Weather	-	

DATA SELECTION

Select Time: PEAK ▼

Т	IME RAN	GE
PEAK	-	PM
	PEAK	
16:45	-	17:45

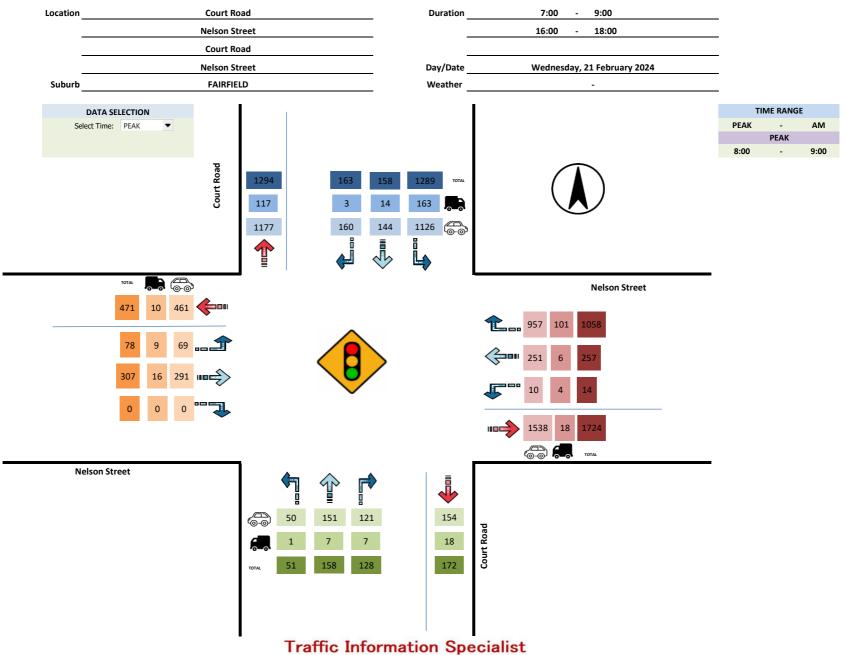
The Horsley Drive



Traffic Information Specialist

ABN: 42 613 389 923 Email info@tistraffic.com.au

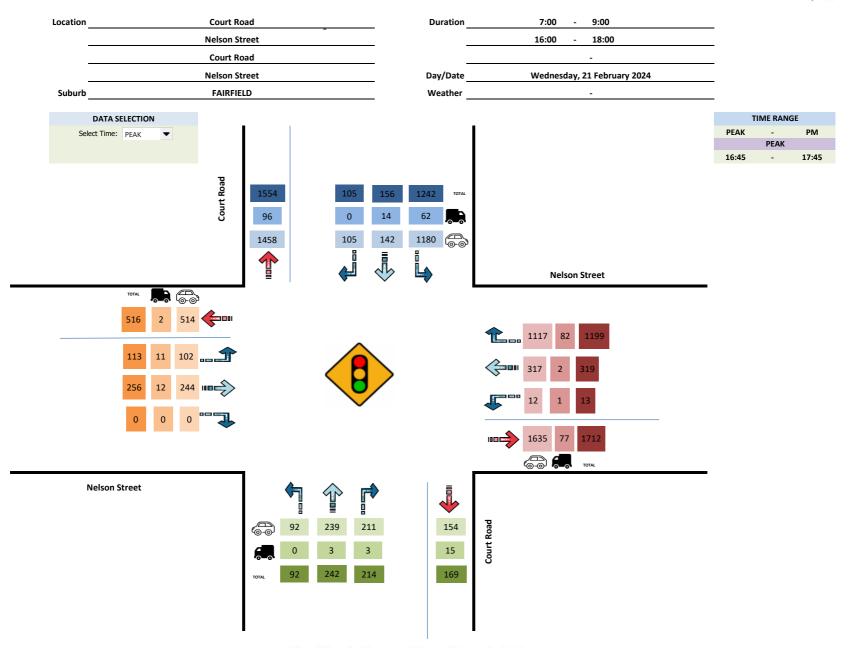




ABN: 42 613 389 923

Email info@tistraffic.com.au

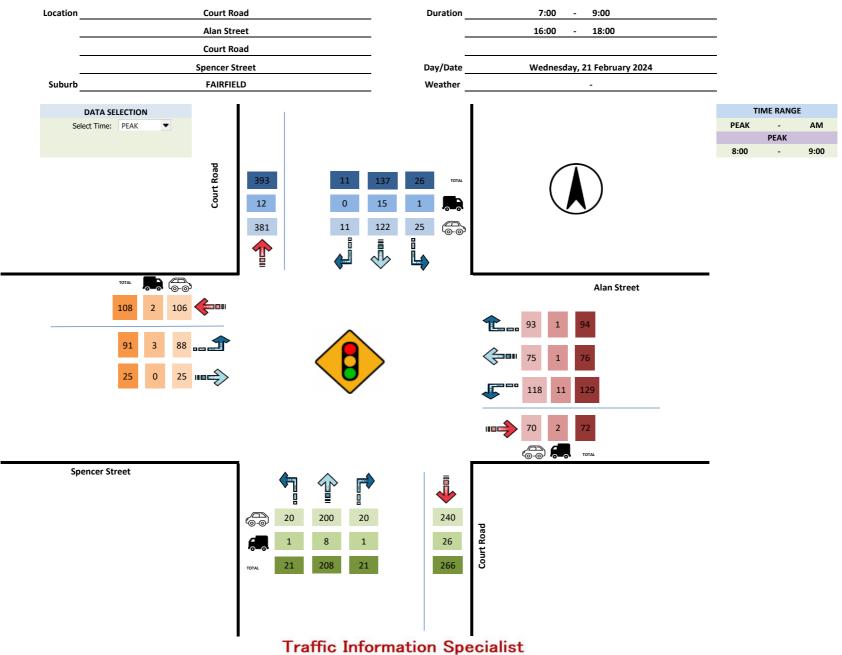




Traffic Information Specialist

ABN: 42 613 389 923 Email info@tistraffic.com.au





ABN: 42 613 389 923 Email info@tistraffic.com.au



Location _		Court Ro	oad			Duration _	7:00 - 9:00			
_		Alan Str	eet	•		_	16:00 - 18:00	<u></u>		
_		Court Ro	oad			_	-			
_		Spencer S	treet			Day/Date _	Wednesday, 21 February 2024			
Suburb _		FAIRFIE	LD			Weather _	<u> </u>			
	DATA SELECTION							,	TIME RANG	SF.
	ect Time: 16:45 ▼		I					16:45	-	17:45
									PEAK	
		Court Road	7 413	23 0 23	195 14 181	26 TOTAL 0 26	Alan Street	16:30	-	17:30
	TOTAL TOTAL		_	•	~	•		-		
	134 0 134 11 1 11						82 1 83 84 0 84 115 12 127			
			1			ĺ	TOTAL	-		
Sį	pencer Street		27 0 TOTAL 27	219 5	26 1 27	296 26 322	Court Road			

Traffic Information Specialist
ABN: 42 613 389 923

Email info@tistraffic.com.au



Attachment 6

SIDRA Result

Site: 1 [The Horsley Drive | Nelson Street | Court Road (Site

Folder: Exsiting Traffic - AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■■ Network: 1 [AM Peak (Network Folder: Existing Traffic)]

AM Peak 8:00am-9:00am Site Category: Base Year

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Vehic	Vehicle Movement Performance														
Mov ID	Turn	Mov Class		lows HV]		rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	Aver. Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Cou	rt Road	V 31 I// 11	,,,	7011/11	70	V/ O			7011					1311/11
1	L2	All MCs	54	2.0	54	2.0	* 0.813	76.4	LOS F	8.1	58.7	1.00	0.96	1.16	19.1
2	T1	All MCs	166	4.4	166	4.4	0.813	70.4	LOS E	8.1	58.7	1.00	0.96	1.17	19.2
3b	R3	All MCs	135	5.5	135	5.5	0.813	75.0	LOS F	7.5	55.1	1.00	0.95	1.18	18.8
Appro	ach		355	4.5	355	4.5	0.813	73.1	LOS F	8.1	58.7	1.00	0.96	1.17	19.0
South	East:	The Hors	ley Driv	/e											
21b	L3	All MCs	15	28.6	15 2	28.6	0.503	26.7	LOS B	5.8	41.9	0.87	0.78	0.87	26.1
21a	L1	All MCs	271	2.3	271	2.3	0.503	24.7	LOS B	5.8	41.9	0.87	0.78	0.87	31.3
23a	R1	All MCs	1114	9.5	1114	9.5	0.621	28.9	LOS C	16.7	126.1	0.78	0.77	0.78	30.3
Appro	ach		1399	8.4	1399	8.4	0.621	28.0	LOS B	16.7	126.1	0.80	0.77	0.80	30.5
North	: The	Horsley D	rive												
7a	L1	All MCs	1357	12.6	1357 1	12.6	* 0.823	42.8	LOS D	25.7	199.4	0.92	0.88	0.94	28.7
8	T1	All MCs	166	8.9	166	8.9	0.390	64.3	LOS E	6.0	44.9	0.89	0.74	0.89	19.6
9	R2	All MCs	172	1.8	172	1.8	0.390	51.9	LOS D	6.0	44.9	0.89	0.78	0.89	25.5
Appro	ach		1695	11.2	1695	11.2	0.823	45.9	LOS D	25.7	199.4	0.91	0.85	0.93	26.1
West:	Nelso	on Street													
10	L2	All MCs	82	11.5	82 1	11.5	0.689	30.9	LOS C	7.8	58.4	0.94	0.81	0.95	25.1
12a	R1	All MCs	323	5.2	323	5.2	* 0.689	60.3	LOS E	7.8	58.4	0.95	0.81	0.96	25.1
Appro	ach		405	6.5	405	6.5	0.689	54.4	LOS D	7.8	58.4	0.95	0.81	0.96	25.1
All Ve	hicles		3854	9.0	3854	9.0	0.823	42.8	LOS D	25.7	199.4	0.88	0.83	0.91	26.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed					
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec					
South: Court Roa	ad														
P1 Full	53	29.5	LOS C	0.1	0.1	0.92	0.92	183.3	200.0	1.09					
North: The Horsl	ey Drive														

P31 Stage 1	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
P32 Stage 2	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
West: Nelson Stre	et									
P4 Full	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
All Pedestrians	211	55.6	LOS E	0.2	0.2	0.95	0.95	209.4	200.0	0.96

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:03:05 AM
Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015 - 46-54 Court Road, Fairfield - V1.1.sip9

Site: 2 [Court Road | Alan Street | Spencer Street (Site Folder:

Exsiting Traffic - AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: 1 [AM Peak (Network Folder: Existing Traffic)]

AM Peak 8:00am-9:00am Site Category: Base Year

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 55 seconds (Site User-Given Cycle Time)

Vehi	Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Dem Fl	nand lows		rival ows	Deg. Satn	Aver. Delay	Level of Service	Aver. Back	Of Queue	e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h		[Total		v/c			[Veh. veh	Dist]		Rate	Cycles	levan /la
South	n: Cou	rt Road	ven/n	70	veh/h	%	V/C	sec		ven	m	_	_		km/h
1		All MCs	22	4.8	22	4.8	0.255	20.8	LOS B	1.8	13.0	0.81	0.66	0.81	33.4
2	T1	All MCs	219		219		0.255	16.3	LOS B	1.8	13.0	0.81	0.66	0.81	29.2
3		All MCs		4.8		4.8	0.255	22.2	LOS B	1.6	11.7	0.80	0.66	0.80	33.3
Appro			263		263		0.255	17.2	LOS B	1.8	13.0	0.81	0.66	0.81	30.3
East:	Alan S	Street													
4	L2	All MCs	136	8.5	136	8.5	0.164	12.7	LOSA	1.3	9.7	0.60	0.67	0.60	34.8
5	T1	All MCs	80	1.3	80	1.3	0.274	9.0	LOS A	1.9	13.1	0.71	0.64	0.71	35.3
6	R2	All MCs	99	1.1	99	1.1	* 0.274	14.2	LOSA	1.9	13.1	0.71	0.64	0.71	31.8
Appro	oach		315	4.3	315	4.3	0.274	12.3	LOSA	1.9	13.1	0.67	0.65	0.67	34.4
North	: Cour	t Road													
7	L2	All MCs	27	3.8	27	3.8	0.069	12.0	LOS A	0.5	3.5	0.68	0.61	0.68	32.3
8	T1	All MCs	144	10.9	144	10.9	* 0.279	16.8	LOS B	1.8	13.9	0.80	0.66	0.80	31.9
9	R2	All MCs	12	0.0	12	0.0	0.279	22.6	LOS B	1.8	13.9	0.81	0.66	0.81	31.4
Appro	oach		183	9.2	183	9.2	0.279	16.5	LOS B	1.8	13.9	0.78	0.65	0.78	31.9
West	: Spen	cer Stree	t												
10	L2	All MCs	96	3.3	96	3.3	* 0.223	18.3	LOS B	1.1	7.8	0.70	0.68	0.70	28.7
11	T1	All MCs	26	0.0	26	0.0	0.053	13.1	LOSA	0.3	1.8	0.61	0.44	0.61	35.0
Appro	oach		122	2.6	122	2.6	0.223	17.2	LOS B	1.1	7.8	0.68	0.63	0.68	30.6
All Ve	ehicles		883	5.0	883	5.0	0.279	15.3	LOS B	1.9	13.9	0.73	0.65	0.73	32.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	Pedestrian Movement Performance														
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed					
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec					
South: Court Roa	ad														
P1 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14					
East: Alan Street															

P2 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14
North: Court Road										
P3 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14
West: Spencer Str	eet									
P4 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14
All Pedestrians	211	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:03:05 AM

Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015 - 46-54 Court Road, Fairfield - V1.1.sip9

Site: 1 [The Horsley Drive | Nelson Street | Court Road (Site

Folder: Existing Traffic - PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■■ Network: 1 [PM Peak (Network Folder: Existing Traffic)]

PM Peak 16:45pm-17:45pm Site Category: Base Year

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Vehic	Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Dem Fl	and ows		rival ows	Deg. Satn	Aver. Delav	Level of Service	Aver. Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total l		[Total veh/h	HV]	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	: Cou	rt Road													
1	L2	All MCs	97	0.0	97	0.0	* 0.857	73.1	LOS F	13.4	94.7	1.00	0.99	1.17	19.5
2	T1	All MCs	255	1.2	255	1.2	0.857	67.3	LOS E	13.4	94.7	1.00	0.99	1.17	21.9
3b	R3	All MCs	225	1.4	225	1.4	0.857	72.2	LOS F	12.3	87.4	1.00	0.99	1.18	21.2
Appro	ach		577	1.1	577	1.1	0.857	70.2	LOS E	13.4	94.7	1.00	0.99	1.17	21.2
South	East:	The Hors	ley Driv	/e											
21b	L3	All MCs	14	7.7	14	7.7	0.589	28.8	LOS C	6.5	45.9	0.90	0.82	0.90	32.1
21a	L1	All MCs	336	0.6	336	0.6	0.589	26.7	LOS B	6.5	45.9	0.90	0.82	0.90	35.4
23a	R1	All MCs	1262	6.8	1262	6.8	0.794	40.6	LOS C	22.5	167.0	0.93	0.87	0.93	35.4
Appro	ach		1612	5.6	1612	5.6	0.794	37.6	LOS C	22.5	167.0	0.92	0.86	0.92	35.3
North	: The	Horsley D	rive												
7a	L1	All MCs	1307	5.0	1307	5.0	* 0.867	57.8	LOS E	28.1	205.3	0.98	0.93	1.05	32.5
8	T1	All MCs	164	9.0	164	9.0	0.542	79.7	LOS F	6.4	48.2	0.97	0.80	0.97	20.2
9	R2	All MCs	111	0.0	111	0.0	0.362	62.1	LOS E	4.2	29.1	0.93	0.78	0.93	26.5
Appro	ach		1582	5.1	1582	5.1	0.867	60.4	LOS E	28.1	205.3	0.98	0.91	1.03	28.6
West:	Nelso	on Street													
10	L2	All MCs	119	9.7	119	9.7	0.639	39.3	LOS C	7.5	55.6	0.92	0.79	0.92	27.8
12a	R1	All MCs	269	4.7	269	4.7	* 0.639	59.2	LOS E	7.5	55.6	0.93	0.79	0.93	27.8
Appro	ach		388	6.2	388	6.2	0.639	53.1	LOS D	7.5	55.6	0.93	0.79	0.93	27.8
All Ve	hicles		4159	4.8	4159	4.8	0.867	52.2	LOS D	28.1	205.3	0.95	0.89	1.00	29.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	Pedestrian Movement Performance													
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	QUE	UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed				
Courtly Court Do	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec				
South: Court Roa	au													
P1 Full	53	29.5	LOS C	0.1	0.1	0.92	0.92	183.3	200.0	1.09				
North: The Horsl	ey Drive													

P31 Stage 1	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
P32 Stage 2	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
West: Nelson Stre	et									
P4 Full	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
All Pedestrians	211	55.6	LOS E	0.2	0.2	0.95	0.95	209.4	200.0	0.96

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:19:32 AM
Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015 - 46-54 Court Road, Fairfield - V1.1.sip9

Site: 2 [Court Road | Alan Street | Spencer Street (Site Folder:

Existing Traffic - PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

■■ Network: 1 [PM Peak (Network Folder: Existing Traffic)]

PM Peak 16:45pm-17:45pm Site Category: Base Year

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 60 seconds (Site User-Given Cycle Time)

Vehi	cle M	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	[Total	lows HV]	FI Total]		Deg. Satn	Aver. Delay	Level of Service	Aver. Back	Dist]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	n: Cou	rt Road	veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
1	L2	All MCs	28	0.0	28	0.0	0.228	19.2	LOS B	2.0	14.1	0.73	0.62	0.73	34.0
2	T1	All MCs	236	2.2	236	2.2	0.228	14.2	LOSA	2.0	14.1	0.74	0.62	0.74	30.1
3	R2	All MCs	28	3.7	28	3.7	0.228	21.1	LOS B	1.8	12.5	0.74	0.63	0.74	33.7
Appro	oach		293	2.2	293	2.2	0.228	15.4	LOS B	2.0	14.1	0.74	0.62	0.74	31.2
East:	Alan S	Street													
4	L2	All MCs	134	9.4	134	9.4	0.192	16.6	LOS B	1.6	12.0	0.68	0.70	0.68	33.6
5	T1	All MCs	88	0.0	88	0.0	0.321	12.4	LOSA	2.3	16.0	0.79	0.68	0.79	34.1
6	R2	All MCs	87	1.2	87	1.2	* 0.321	19.3	LOS B	2.3	16.0	0.79	0.68	0.79	29.8
Appro	oach		309	4.4	309	4.4	0.321	16.1	LOS B	2.3	16.0	0.75	0.69	0.75	33.1
North	: Cour	t Road													
7	L2	All MCs	27	0.0	27	0.0	0.079	11.0	LOSA	0.7	4.7	0.65	0.57	0.65	32.7
8	T1	All MCs	205	7.2	205	7.2	* 0.319	15.0	LOS B	2.6	19.3	0.74	0.63	0.74	32.5
9	R2	All MCs	24	0.0	24	0.0	0.319	20.5	LOS B	2.6	19.3	0.76	0.64	0.76	32.0
Appro	oach		257	5.7	257	5.7	0.319	15.1	LOS B	2.6	19.3	0.73	0.63	0.73	32.5
West	: Spen	cer Stree	t												
10	L2	All MCs	119	0.9	119	0.9	* 0.322	22.5	LOS B	1.7	11.7	0.78	0.72	0.78	27.0
11	T1	All MCs	45	0.0	45	0.0	0.099	15.8	LOS B	0.5	3.7	0.66	0.49	0.66	34.1
Appro	oach		164	0.6	164	0.6	0.322	20.6	LOS B	1.7	11.7	0.75	0.66	0.75	29.6
All Ve	ehicles		1023	3.5	1023	3.5	0.322	16.4	LOS B	2.6	19.3	0.74	0.65	0.74	31.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	Pedestrian Movement Performance													
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE [Ped		Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed				
	ped/h	sec		ped	m m		rtato	sec	m	m/sec				
South: Court Roa	ad													
P1 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12				
East: Alan Street														

P2 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12
North: Court Road										
P3 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12
West: Spencer Str	eet									
P4 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12
All Pedestrians	211	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:19:32 AM

Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015 - 46-54 Court Road, Fairfield - V1.1.sip9

Site: 1 [The Horsley Drive | Nelson Street | Court Road (Site

Folder: Post Development - AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: 1 [AM Peak (Network Folder: Post Development)]

AM Peak 8:00am-9:00am

Site Category: Future Conditions 1

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Vehicle Movement Performance Mov Turn Mov Demand Arrival Deg. Aver. Level of Aver. Back Of Queue Prop. Eff. Aver. Aver.															
Mov ID	Turn	Mov Class	FI	ows	Flo	ows	Deg. Satn	Aver. Delay	Level of Service			e Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total I veh/h		[Total l veh/h	-1V] 	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
South	n: Cou	rt Road	VO11/11	70	VO11/11	/0	V/ O	300		VOIT					KIII/II
1	L2	All MCs	80	1.3	80	1.3	* 0.837	75.6	LOS F	9.8	70.6	1.00	0.98	1.18	19.1
2	T1	All MCs	193	3.8	193	3.8	0.837	69.9	LOS E	9.8	70.6	1.00	0.98	1.18	19.2
3b	R3	All MCs	156	4.7	156	4.7	0.837	74.5	LOS F	9.2	67.2	1.00	0.97	1.19	18.9
Appro	ach		428	3.7	428	3.7	0.837	72.6	LOS F	9.8	70.6	1.00	0.98	1.18	19.1
South	East:	The Hors	ley Driv	/e											
21b	L3	All MCs	15	28.6	15 2	28.6	0.515	26.9	LOS B	5.6	40.4	0.88	0.78	0.88	26.0
21a	L1	All MCs	271	2.3	271	2.3	0.515	24.9	LOS B	5.6	40.4	0.88	0.78	0.88	31.3
23a	R1	All MCs	1114	9.5	1114	9.5	0.649	31.2	LOS C	17.4	131.7	0.82	0.79	0.82	29.7
Appro	oach		1399	8.4	1399	8.4	0.649	30.0	LOS C	17.4	131.7	0.83	0.79	0.83	30.0
North	: The	Horsley D	rive												
7a	L1	All MCs	1357	12.6	1357 1	12.6	* 0.863	51.0	LOS D	28.5	220.8	0.96	0.93	1.02	27.1
8	T1	All MCs	174	8.5	174	8.5	0.424	68.3	LOS E	6.3	47.0	0.91	0.75	0.91	19.2
9	R2	All MCs	172	1.8	172	1.8	0.423	53.7	LOS D	6.2	44.0	0.91	0.79	0.91	25.2
Appro	oach		1702	11.1	1702 1	11.1	0.863	53.0	LOS D	28.5	220.8	0.95	0.90	1.00	24.7
West	Nelso	on Street													
10	L2	All MCs	82	11.5	82 ′	11.5	0.689	32.1	LOS C	7.8	58.4	0.94	0.81	0.95	25.1
12a	R1	All MCs	323	5.2	323	5.2	* 0.689	60.0	LOS E	7.8	58.4	0.95	0.81	0.96	25.1
Appro	oach		405	6.5	405	6.5	0.689	54.4	LOS D	7.8	58.4	0.95	0.81	0.96	25.1
All Ve	hicles		3935	8.9	3935	8.9	0.863	47.1	LOS D	28.5	220.8	0.91	0.86	0.95	25.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	Pedestrian Movement Performance													
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	UE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed				
	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec				
South: Court Roa	ad													
P1 Full	53	29.4	LOS C	0.1	0.1	0.92	0.92	183.2	200.0	1.09				
North: The Horsl	ey Drive													

P31 Stage 1	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
P32 Stage 2	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
West: Nelson Stre	et									
P4 Full	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
All Pedestrians	211	55.5	LOS E	0.2	0.2	0.95	0.95	209.4	200.0	0.96

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:03:09 AM
Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015-V1.1-SIDRA\24015-V1.1.sip9

Site: 2 [Court Road | Alan Street | Spencer Street (Site Folder:

Post Development - AM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: 1 [AM Peak (Network Folder: Post Development)]

AM Peak 8:00am-9:00am

Site Category: Future Conditions 1

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 55 seconds (Site User-Given Cycle Time)

Vehi	cle M	ovemen	t Perfo	rma	nce										
Mov ID	Turn	Mov Class		lows HV]		rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	Aver. Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	n: Cou	rt Road	VCII/II	70	VCII/II	/0	V/C	300		VCII	- '''				IXIII/II
1	L2	All MCs	22	4.8	22	4.8	0.264	20.9	LOS B	1.9	13.5	0.81	0.66	0.81	33.4
2	T1	All MCs	229	3.7	229	3.7	0.264	16.4	LOS B	1.9	13.5	0.81	0.66	0.81	29.2
3	R2	All MCs	22	4.8	22	4.8	0.264	22.3	LOS B	1.7	12.2	0.81	0.66	0.81	33.3
Appro	oach		274	3.8	274	3.8	0.264	17.2	LOS B	1.9	13.5	0.81	0.66	0.81	30.2
East:	Alan S	Street													
4	L2	All MCs	136	8.5	136	8.5	0.164	12.7	LOSA	1.3	9.7	0.60	0.67	0.60	34.8
5	T1	All MCs	80	1.3	80	1.3	0.289	9.1	LOSA	2.0	13.8	0.72	0.65	0.72	35.3
6	R2	All MCs	107	1.0	107	1.0	* 0.289	14.3	LOSA	2.0	13.8	0.72	0.65	0.72	31.7
Appro	oach		323	4.2	323	4.2	0.289	12.3	LOSA	2.0	13.8	0.67	0.66	0.67	34.3
North	: Cour	t Road													
7	L2	All MCs	27	3.8	27	3.8	0.072	12.3	LOSA	0.5	3.7	0.70	0.61	0.70	32.1
8	T1	All MCs	152	10.4	152	10.4	* 0.290	17.0	LOS B	1.9	14.5	0.80	0.66	0.80	31.9
9	R2	All MCs	12	0.0	12	0.0	0.290	22.7	LOS B	1.9	14.5	0.82	0.67	0.82	31.4
Appro	oach		191	8.8	191	8.8	0.290	16.7	LOS B	1.9	14.5	0.79	0.65	0.79	31.9
West	: Spen	cer Stree	t												
10	L2	All MCs	96	3.3	96	3.3	* 0.223	18.3	LOS B	1.1	7.8	0.70	0.68	0.70	28.7
11	T1	All MCs	26	0.0	26	0.0	0.053	13.1	LOSA	0.3	1.8	0.61	0.44	0.61	35.0
Appro	oach		122	2.6	122	2.6	0.223	17.2	LOS B	1.1	7.8	0.68	0.63	0.68	30.6
All Ve	ehicles		909	4.9	909	4.9	0.290	15.4	LOS B	2.0	14.5	0.74	0.66	0.74	32.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Mo	Pedestrian Movement Performance													
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE [Ped		Prop. Que	Eff. Stop Rate	Travel Time	Travel Dist.	Aver. Speed				
	ped/h	sec		ped	m			sec	m	m/sec				
South: Court Roa	ad													
P1 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14				
East: Alan Street	t													

P2 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14
North: Court Road										
P3 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14
West: Spencer Str	eet									
P4 Full	53	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14
All Pedestrians	211	21.9	LOS C	0.1	0.1	0.89	0.89	175.7	200.0	1.14

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:03:09 AM

Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015-V1.1-SIDRA\24015-V1.1.sip9

Site: 1 [The Horsley Drive | Nelson Street | Court Road (Site

Folder: Post Development - PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: 1 [PM Peak (Network Folder: Post Development)]

PM Peak 16:45pm-17:45pm Site Category: Future Conditions 1

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Vehic	cle M	ovement	Perfo	rma	nce										
Mov ID	Turn	Mov Class	Dem Fl	and ows		rival ows	Deg. Satn	Aver. Delav	Level of Service	Aver. Back	Of Queue	Prop. Que	Eff. Stop	Aver. No. of	Aver. Speed
			[Total veh/h		[Total l veh/h		v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	' km/h
South	: Cou	rt Road													
1	L2	All MCs	103	0.0	103	0.0	* 0.857	72.3	LOS F	13.9	97.9	1.00	0.99	1.16	19.6
2	T1	All MCs	261	1.2	261	1.2	0.857	66.6	LOS E	13.9	97.9	1.00	0.99	1.16	22.0
3b	R3	All MCs	234	1.4	234	1.4	0.857	71.4	LOS F	12.7	90.3	1.00	0.98	1.18	21.4
Appro	ach		598	1.1	598	1.1	0.857	69.5	LOS E	13.9	97.9	1.00	0.99	1.17	21.3
South	East:	The Hors	ley Driv	/e											
21b	L3	All MCs	14	7.7	14	7.7	0.618	29.8	LOS C	6.7	47.0	0.91	0.82	0.91	31.6
21a	L1	All MCs	336	0.6	336	0.6	0.618	27.7	LOS B	6.7	47.0	0.91	0.82	0.91	35.0
23a	R1	All MCs	1262	6.8	1262	6.8	0.807	42.4	LOS C	23.1	171.4	0.94	0.89	0.95	34.8
Appro	ach		1612	5.6	1612	5.6	0.807	39.2	LOS C	23.1	171.4	0.94	0.87	0.95	34.8
North	: The	Horsley D	rive												
7a	L1	All MCs	1307	5.0	1307	5.0	* 0.888	62.4	LOS E	29.8	217.3	1.00	0.96	1.10	31.3
8	T1	All MCs	186	7.9	186	7.9	0.586	79.9	LOS F	7.3	54.4	0.97	0.81	0.97	20.3
9	R2	All MCs	111	0.0	111	0.0	0.347	61.1	LOS E	4.1	28.8	0.93	0.78	0.93	26.7
Appro	ach		1604	5.0	1604	5.0	0.888	64.4	LOS E	29.8	217.3	0.99	0.93	1.07	27.6
West:	Nelso	on Street													
10	L2	All MCs	119	9.7	119	9.7	0.639	38.8	LOS C	7.5	55.6	0.92	0.79	0.92	27.8
12a	R1	All MCs	269	4.7	269	4.7	* 0.639	59.5	LOS E	7.5	55.6	0.93	0.79	0.93	27.8
Appro	ach		388	6.2	388	6.2	0.639	53.1	LOS D	7.5	55.6	0.93	0.79	0.93	27.8
All Ve	hicles		4202	4.8	4202	4.8	0.888	54.4	LOS D	29.8	217.3	0.96	0.90	1.02	29.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Movement Performance													
Mov ID Crossing	Dem. Flow			AVERAGE QUE	Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed				
Country Count Do	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec			
South: Court Roa	au												
P1 Full	53	29.4	LOS C	0.1	0.1	0.92	0.92	183.3	200.0	1.09			
North: The Horsl	North: The Horsley Drive												

P31 Stage 1	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
P32 Stage 2	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
West: Nelson Stre	et									
P4 Full	53	64.3	LOS F	0.2	0.2	0.96	0.96	218.1	200.0	0.92
All Pedestrians	211	55.6	LOS E	0.2	0.2	0.95	0.95	209.4	200.0	0.96

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:19:42 AM
Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015-V1.1-SIDRA\24015-V1.1.sip9

Site: 2 [Court Road | Alan Street | Spencer Street (Site Folder:

Post Development - PM Peak)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: 1 [PM Peak (Network Folder: Post Development)]

PM Peak 16:45pm-17:45pm Site Category: Future Conditions 1

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 60 seconds (Site User-Given Cycle Time)

Vehicle Movement Performance															
Mov ID	Turn	Mov Class		lows HV]		rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	Aver. Back [Veh. veh	Of Queue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	n: Cou	rt Road	V 31 I// 11	,,,	731,711	,,	V/ O			7011					1(11)/11
1	L2	All MCs	28	0.0	28	0.0	0.257	20.1	LOS B	2.2	15.6	0.75	0.63	0.75	33.8
2	T1	All MCs	259	2.0	259	2.0	0.257	15.2	LOS B	2.2	15.6	0.76	0.64	0.76	29.6
3	R2	All MCs	28	3.7	28	3.7	0.257	22.2	LOS B	2.0	14.0	0.77	0.64	0.77	33.5
Appro	oach		316	2.0	316	2.0	0.257	16.3	LOS B	2.2	15.6	0.76	0.64	0.76	30.7
East:	Alan S	Street													
4	L2	All MCs	134	9.4	134	9.4	0.184	15.9	LOS B	1.5	11.6	0.67	0.69	0.67	33.9
5	T1	All MCs	88	0.0	88	0.0	0.344	11.8	LOSA	2.5	17.4	0.79	0.69	0.79	34.2
6	R2	All MCs	105	1.0	105	1.0	* 0.344	18.6	LOS B	2.5	17.4	0.79	0.69	0.79	29.9
Appro	oach		327	4.2	327	4.2	0.344	15.6	LOS B	2.5	17.4	0.74	0.69	0.74	33.1
North	: Cour	t Road													
7	L2	All MCs	27	0.0	27	0.0	0.084	11.1	LOS A	0.7	4.9	0.67	0.58	0.67	32.4
8	T1	All MCs	208	7.1	208	7.1	* 0.340	15.9	LOS B	2.7	20.1	0.76	0.65	0.76	32.2
9	R2	All MCs	24	0.0	24	0.0	0.340	21.5	LOS B	2.7	20.1	0.78	0.66	0.78	31.7
Appro	oach		260	5.7	260	5.7	0.340	15.9	LOS B	2.7	20.1	0.75	0.64	0.75	32.2
West	: Spen	cer Stree	t												
10	L2	All MCs	119	0.9	119	0.9	* 0.322	22.5	LOS B	1.7	11.7	0.78	0.72	0.78	27.0
11	T1	All MCs	45	0.0	45	0.0	0.099	15.8	LOS B	0.5	3.7	0.66	0.49	0.66	34.1
Appro	oach		164	0.6	164	0.6	0.322	20.6	LOS B	1.7	11.7	0.75	0.66	0.75	29.6
All Ve	ehicles		1067	3.4	1067	3.4	0.344	16.6	LOS B	2.7	20.1	0.75	0.66	0.75	31.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Pedestrian Movement Performance												
Mov ID Crossing	Dem. Flow			AVERAGE BACK OF QUEUE		Prop. Que	Eff. Stop	Travel Time	Travel Dist.	Aver. Speed		
South: Court Roa	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	m/sec		
South. Court No.	au											
P1 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12		
East: Alan Street												

P2 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12
North: Court Road										
P3 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12
West: Spencer Str	eet									
P4 Full	53	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12
All Pedestrians	211	24.4	LOS C	0.1	0.1	0.90	0.90	178.2	200.0	1.12

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: GENESIS TRAFFIC | Licence: NETWORK / 1PC | Processed: Tuesday, 27 February 2024 9:19:42 AM

Project: C:\Users\Genesis Traffic\OneDrive - Genesis Traffic Solutions\Documents - Genesis Projects\2024\24015 - 55 Court Road, Fairfield (Amended DA)\Model\24015-V1.1-SIDRA\24015-V1.1.sip9





Better Developments with Genesis Traffic